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Completion Report for Regional Aquifer Well R-44



Prepared by the Environmental Programs Directorate

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May 2009

Responsible project leader:

Mark Everett	Ulach lundt	Project Leader	Environmental Programs	5-26-09
Printed Name	Signature	Title	Organization	Date

Responsible LANS representative:

Michael Graham	Oplangen	Associate Director	Environmental Programs	5/27/09
Printed Name	Signature	Title	Organization	Date

Responsible DOE representative:

R Chegy Signature Project Director David R. Gregory 09 DOE-LASO Printed Name Title Organization Date

EXECUTIVE SUMMARY

This well completion report describes the drilling, installation, development, and aquifer testing of Los Alamos National Laboratory's regional aquifer well R-44, which is located in a tributary of Mortandad Canyon, Technical Area 05 (TA-05) in Los Alamos County, New Mexico. This report was written in accordance with the requirements in Section IV.A.3.e.iv of the March 1, 2005, Compliance Order on Consent. The well was installed at the direction of the New Mexico Environment Department (NMED) to monitor groundwater quality and contaminant movement and to define the southern limit of chromium contamination in the vicinity of well R-28 (which has consistently shown elevated concentrations of chromium in the regional aquifer at the Laboratory). The well will also be used to monitor water levels within the regional aquifer and measure pumping effects from nearby water supply wells.

The R-44 borehole was drilled using dual-rotary air-drilling methods. Fluid additives used included potable water and foam. Foam-assisted drilling was used only in the vadose zone and ceased approximately 100 ft above the regional aquifer; no drilling-fluid additives other than small amounts of potable water were used in the regional aquifer. Additive-free drilling provides minimal impacts to the groundwater and the formation. The R-44 borehole was successfully completed to total depth using dual-rotary casing-advance and open-hole drilling methods.

A retractable 16-in. casing was advanced through the Bandelier Tuff, Guaje Pumice Bed and basaltic volcaniclastic sediments to a depth of 345.8 ft bgs. A 15-in. open borehole was advanced with fluid-assisted air-rotary methods with a downhole hammer bit through the Cerros del Rio basalt and into the Puye Formation to a depth of 765 ft bgs. Then 12-in. casing was advanced with an 11-5/8-in. tricone bit through the remainder of the Puye Formation, through Miocene pumiceous sediments, and through Santa Fe Group Miocene riverine gravels to a total depth of 1094 ft bgs.

Well R-44 was completed as a dual-screen well to evaluate water quality and measure water levels at two discrete depth intervals within the regional aquifer. Well screens will be separated by a packer, as part of the permanent dedicated sampling system, to ensure isolation of each groundwater bearing zone. The upper 10-ft long screened interval has the top of the screen set at 895 ft bgs and the lower 10-ft long screened interval has the top of the screen set at 985.3 ft bgs. Both screen intervals are within the Puye Formation. The composite depth to water after well installation and well development was 879.1 ft bgs.

The well was completed in accordance with an NMED-approved well design and was developed and met target water-quality parameters. Hydrogeologic testing indicated that monitoring well R-44 is highly productive and will perform effectively to meet the planned objectives. Water-level transducers will be placed in the upper and lower well screens in the R-44 well, and groundwater sampling will be performed as part of the facility-wide groundwater-monitoring program.

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Acronyms and Abbreviations

µS/cm	microsiemens per centimeter
amsl	above mean sea level
BETCO	barometric and Earth tide correction
bgs	below ground surface
CNL	Compensated Neutron Log
Consent Order	Compliance Order on Consent
cu	capture unit
DO	dissolved oxygen
ECS	Elemental Capture Sonde
EES-14	Earth and Environmental Sciences Group
ENV-MAQ	Environmental Division–Meteorology and Air Quality Group

EP	Environmental Programs		
gAPI	American Petroleum Institute gamma ray		
GR	gamma ray		
HNGS	Hostile Natural Gamma Spectroscopy		
IC	ion chromatography		
ICPMS	inductively coupled (argon) plasma mass spectrometry		
ICPOES	inductively coupled (argon) plasma mass spectrometry		
ID	identification		
I.D.	inside diameter		
ICPMS	inductively coupled (argon) plasma mass spectrometry		
ICPOES	inductively coupled (argon) plasma optical emission spectroscopy		
LANL	Los Alamos National Laboratory		
lbf	pound force		
MDA	material disposal area		
mV	millivolt		
NMED	New Mexico Environment Department		
NTU	nephelometric turbidity unit		
O.D.	outside diameter		
ORP	oxidation-reduction potential		
PVC	polyvinyl chloride		
RPF	Records Processing Facility		
SOP	standard operating procedure		
SVOC	semivolatile organic compound		
ТА	technical area		
TD	total depth		
TLD	Triple Detector Litho-Density		
TOC	total organic carbon		
VOC	volatile organic compound		
WCSF	waste characterization strategy form		
WES-EDA	Waste and Environmental Services Division-Environmental Data and Analysis		

1.0 INTRODUCTION

This completion report summarizes the site preparation, drilling, well construction, well development, and aquifer testing for regional aquifer well R-44. The report is written in accordance with the requirements in Section IV.A.3.e.iv of the March 1, 2005, Compliance Order on Consent (the Consent Order). Well R-44 was drilled from November 10, 2008 to December 8, 2008, and the well was completed from December 13, 2009, to January 15, 2009, at Los Alamos National Laboratory (LANL or the Laboratory) for the Environmental Programs (EP) Directorate Water Stewardship Program.

The R-44 project site is located in a small tributary of Mortandad Canyon in the vicinity of regional well R-13 within Technical Area 05 (TA-05), Los Alamos County, New Mexico (Figure 1.0-1). The purposes of the R-44 monitoring well are to monitor potential releases of contaminants from Mortandad and Sandia Canyon sources, assess the conceptual model for contaminant fate and transport of known chromium contamination in the vicinity of well R-28, monitor water levels within the regional aquifer, and measure pumping effects from water-supply well PM-5 and other wells in the vicinity.

The primary objective of the drilling activities at R-44 was to drill and install a dual-screen regional aquifer monitoring well in the uppermost part of the regional groundwater system. The two-screen approach was designed to determine the vertical extent of potential chromium contamination so that pathways and potential future impacts to regional groundwater may be assessed. Water-level transducers will be placed in upper and lower well screens to evaluate hydraulic connections between this monitoring well, other monitoring wells and nearby water-supply well PM-5. Secondary objectives were to collect drill-cutting samples, conduct borehole geophysical logging, and investigate potential perched groundwater zones.

The R-44 borehole was drilled to a total depth (TD) of 1094.0 ft below ground surface (bgs). A monitoring well was installed with two screens. Currently, a temporary packer is being used to isolate the two well screens until the permanent sampling system that is being built by an off-site contractor can be installed. The permanent sampling system will isolate the two screens with a packer when installed in the near future. The upper 10-ft long screened interval is between 895.0 and 905.0 ft bgs and the lower 10-ft long screened interval is between 895.0 and 905.0 ft bgs and the lower 10-ft long screened interval is between 9, 2008. Cuttings samples were collected at 5-ft intervals in the borehole from ground surface to TD. Post installation activities included well development, aquifer testing, surface completion, and a geodetic survey. Future activities include dedicated sampling system installation, site restoration, and waste management.

The information presented in this report was compiled from field reports and daily activity summaries. Records, including field reports, field logs, and survey information, are on file at the Laboratory's Records Processing Facility (RPF). This report contains brief descriptions of activities and supporting figures, tables, and appendixes completed to date associated with the R-44 project.

2.0 PRELIMINARY ACTIVITIES

Preliminary activities included preparing administrative planning documents and preparing the drill pad. All preparatory activities were completed in accordance with Laboratory policies and procedures and regulatory requirements.

2.1 Administrative Preparation

The following documents helped guide the implementation of the scope of work for well R-44: "Final Drilling Plan for Regional Aquifer Wells R-44 and R-45" (TerranearPMC 2008, 105083); "Integrated Work Document for Regional and Intermediate Aquifer Well Drilling" (LANL 2007, 100972); "Storm Water Pollution Prevention Plan Addendum" (LANL 2006, 092600); and "Waste Characterization Strategy Form for the R-38, R-41, R-44, R-45, and R-46 Regional Groundwater Well Installation and Corehole Drilling" (LANL 2008, 103916).

2.2 Site Preparation

Site preparation was performed by LANL staff several weeks prior to rig mobilization. Between November 8 and 9, 2008, activities included mobilizing the drill rig, air compressors, trailers, and support vehicles to the drill site and staging alternative drilling tools and construction materials at the Pajarito Road lay down yard.

Office supply trailers, generators, and general field equipment were moved on-site after mobilization of drilling equipment. Potable water was obtained from the Puye Road fire hydrant and a fire hydrant near the Los Alamos County landfill on East Jemez Road. Safety barriers and signs were installed around the borehole-cuttings containment pit and along the perimeter of the work area.

3.0 DRILLING ACTIVITIES

This section describes the drilling strategy and approach and provides a chronological summary of field activities conducted at monitoring well R-44.

3.1 Drilling Approach

The drilling methodology and selection of equipment, including drill casing sizes, for R-44 were designed to retain the ability to case off perched groundwater and ensure reaching TD with a sufficiently sized casing to allow well installation with the required 2-in. minimum annular filter pack thickness for a 5.56-in.-outside diameter (O.D.) well. It was anticipated that if perched groundwater was encountered at R-44, the perched zone would be isolated and sealed off either with casing or by cementing to avoid commingling perched groundwater with the regional aquifer.

Dual-rotary drilling methods using a Foremost DR-24HD drill rig were employed to drill the R-44 borehole. Dual-rotary drilling has the advantage of simultaneously advancing and casing the borehole. The Foremost DR-24HD drill rig was equipped with conventional drilling rods, tricone bits, downhole hammer bits, one deck-mounted 900 ft³/min air compressor, and general drilling equipment. Auxiliary equipment included two Sullair 1150 ft³/min trailer-mounted air compressors. Two sizes of A53 grade B flush-welded mild carbon-steel casing (16-in. and 12-in. inside-diameter [I.D.]) were used for the R-44 project. The dual-rotary technique used filtered compressed air and fluid-assisted air to evacuate cuttings from the borehole. Cuttings samples were collected at 5-ft intervals in the borehole from ground surface to TD to characterize the hydrostratigraphy of rock units encountered in the borehole.

Drilling fluids, other than air, used in the vadose zone included municipal water and a mixture of municipal water with Baroid AQF-2 foaming agent. The fluids were used to cool the bit and help lift cuttings from the borehole. Use of foaming agents was terminated at 780 ft bgs, approximately 100 ft above the predicted regional aquifer water table. No additives other than municipal water were used for drilling within the

regional aquifer. Total amounts of drilling fluids introduced into the borehole and those recovered are recorded and presented in Table 3.1-1.

3.2 Chronology of Drilling Activities

Mobilization of drilling equipment and supplies to the R-44 site occurred during November 8 and 9, 2008. The borehole was initiated the next day, at 0145 hours using dual-rotary methods with 16-in. casing and a 15-in. tri-cone, long-tooth carbide bit. After drilling and advancing 16-in. casing through the alluvium, Bandelier Tuff, and volcaniclastic sediments overlying the Cerros del Rio basalt, the 16-in. casing was landed at 345.8 ft bgs in the morning of November 12, 2008. In conjunction with preparation to start openhole drilling the top-head drive developed a minor hydraulic leak which required replacing a seal.

Open-hole drilling resumed using a 15-in. hammer bit at the top of Cerros del Rio basalt at 1807 h on November 13, 2008. Drilling progressed smoothly through the basalt to the contact with underlying Puye Formation sediments, at 707.0 ft bgs (at 0400 h on November 16, 2008). Progress was slowed because of problems with one of the two auxiliary Sulair air compressors. However, open-hole drilling was eventually suspended at 765.0 ft bgs (within the Puye Formation sediments) due to borehole instability.

On November 17, 2008 the 16-in. casing was cut in order to detach the welded drive shoe at 344.0 ft bgs, prior to running the Laboratory's video and geophysical (gamma ray and induction) logging tools in the borehole. The video tool revealed water entering the borehole at 739 ft bgs. Two groundwater samples were collected from this depth using a bailer; however the exact depth of this water was suspect because there were "knots" in the logging wire line. Because of the lack of water at this depth during later monitoring, and based on chemical analysis of these samples, it was decided that the water was most likely drilling water and not perched groundwater.

The drive shoe and 20-ft sections of 12-in. casing were welded and installed in the hole from November 18 through 24, 2008. Several mechanical and hydraulic problems with the top-head drive required ordering replacement parts and servicing, which slowed progress. On December 2, 2008, dual-rotary drilling commenced with the 12-in. casing and an 11-5/8 in. tricone bit. Drilling was unusually slow in the soft Puye Formation sediments, and the bit was pulled on December 3, 2008, for inspection. Several carbide buttons were observed to be absent from the bit cones, but of particular note was the presence of heavy score marks on the bit just above the cones. The score marks were indicative of the bit cutting into the steel drive shoe due to the bit not being able to be advanced outside the bottom of the 12-in. casing. All signs pointed to a misalignment somewhere toward the bottom of the 12-in. casing string. The decision to remove the entire 12-in. casing string for inspection was determined to be imperative.

Once on the surface, the bottom 12-in. casing joint was found to be slightly bent and the drive shoe weld showed some cracking. A new drive shoe was then welded on to a new lead casing joint on December 3, 2008, and the 12-in. casing string was reinstalled in the borehole. Drilling with dual-rotary methods and 12-in. casing recommenced on December 6, 2008. Eleven groundwater samples were collected, by air-lifting, from the 920 to 1094 ft bgs interval on December 7 and 8, 2008. Significant water production in the borehole was noted beginning at about 990 ft bgs and some indication of formation heaving occurred at 1094 ft bgs. A total depth of 1094 ft bgs was reached on December 8, 2008 at 1735 h. The next day the drill string was tripped out of the hole in preparation for geophysical logging by Schlumberger on December 9, 2008. After logging concluded, a stable depth-to-water of 879.1 ft bgs was recorded the same day.

Twelve-in. (74.0 ft casing and shoe) and 16-in. (1.8 ft casing and shoe) drill casing were left in the borehole. The longer length of 12-in. casing was left in place to help control heaving. The 12-in. casing stub was buried in backfill and isolated by the lowermost bentonite seal and the 16 in. casing stub was set in bentonite to avoid unwanted impacts in future.

Before moving the drilling rig off the site, the 12-in. casing was cut on December 10, 2008, at 1020.0 ft bgs. The rig was moved off site early in the morning of December 11, 2008, to the next drilling location (R-46).

The field crews typically worked two 12-h shifts per day (24-h operation) and 7 d/wk. Daily activities progressed without weather delays throughout the duration of drilling. Only minor mechanical delays with the 12-in. casing shoe, top-head drive, and air compressors slowed drilling progress.

4.0 SAMPLING ACTIVITIES

This section describes the cuttings and groundwater sampling activities at well R-44. All sampling activities were conducted in accordance with applicable quality procedures.

4.1 Cuttings Sampling

Cuttings samples were collected from the R-44 borehole at 5-ft intervals from ground surface to the TD of 1094.0 ft bgs. At each interval, approximately 500 mL of bulk cuttings were collected from the discharge hose, placed in resealable plastic bags, labeled, and archived in core boxes. Sieved fractions (>#10 and >#35 mesh) were also collected from ground surface to bottom depth and placed in chip trays along with unsieved (whole rock) cuttings. Radiation control technicians screened cuttings before removal from the site. The core boxes and chip trays were delivered to the Laboratory's archive at the conclusion of drilling activities. All screening measurements were within the range of background values.

Drilling and sample collection methods used at R-44 did not retain a majority of the fine fraction (silt and clay) of the drill cuttings, and much of the fine material throughout the borehole was lost. The volume of compressed air and water required for circulation made catching samples difficult, and fines were selectively lost during sample collection. Site geologists manually collected samples with a wire mesh basket directly from the discharge hose, and discharge velocities commonly forced the fine fraction of sample through the basket. Recovery of the coarser fraction of the cuttings samples was successful in nearly 100% of the borehole. The borehole lithologic log for R-44 stratigraphy is summarized in section 5.1 and detailed in Appendix A.

4.2 Water Sampling

Groundwater-screening samples were collected from the drilling discharge hose at approximate 20-ft intervals starting at 739 ft bgs to evaluate a potential perched zone (see discussion in section 3.2) and continued through the top of the regional aquifer to the borehole's TD of 1094.0 ft bgs. Typically, upon reaching the bottom of a 20-ft run of casing, the driller would stop water circulation (if injecting water) and circulate air, and as the discharge cleared; a water sample was collected directly from the discharge hose. Not all depth intervals below the top of the regional groundwater table could be captured at the end of each casing run. Alternatively, some water samples were collected upon start-up of the next casing run after the borehole equilibrated. Refer to Table 4.2-1 for a summary of screening samples collected at well R-44.

Eleven groundwater-screening samples, from depths of 739.0 to 1094.0 ft bgs, were collected during drilling operations by bailing or air-lifting water samples through the drill string. Two of these samples represented waters collected while drilling through the vadose zone to evaluate the presence or absence of perched groundwater. Drilling screening samples were analyzed for anions and metals, and one sample was analyzed for tritium.

Four regional groundwater-screening samples were collected during well development; two from the upper screen interval (895–905 ft bgs) and two from the lower screen interval (985.3–995.2 ft bgs). Development screening samples were analyzed for anions, metals, and total organic carbon (TOC).

Twelve regional groundwater-screening samples were collected at regular intervals (approximately one sample per 4 h) during aquifer testing. Six of these screening samples were collected from the upper screen interval (895–905 ft bgs), and six samples were collected from the lower screen interval (985.3–995.2 ft bgs). The groundwater samples were collected from a stainless-steel riser pipe that was connected to the surface discharge line from the submersible pump. Aquifer-testing screening samples were analyzed for dissolved anions, metals and TOC.

Groundwater characterization samples were collected from the completed well in accordance with the Consent Order. The samples were analyzed for the full suite of constituents including radioactive elements; anions/cations; general inorganic chemicals; volatile and semi-volatile organic compounds; and stable isotopes of hydrogen, nitrogen, and oxygen. These groundwater analytical results will be reported in the annual update to the "Interim Facility-Wide Groundwater Monitoring Plan."

5.0 GEOLOGY AND HYDROGEOLOGY

A brief description of the geologic and hydrogeologic features encountered at R-44 is presented below. The Laboratory's geology task leader and site geologists examined cuttings and geophysical logs to determine geologic contacts and hydrogeologic conditions. Drilling observations, video logging, water-level measurements, and geophysical logs were used to characterize groundwater occurrences encountered at R-44.

5.1 Stratigraphy

The stratigraphy for the R-44 borehole is presented below in order of youngest to oldest geologic units. Lithologic descriptions are based on cuttings samples collected from the discharge hose. Cuttings and borehole geophysical logs were used to identify geologic contacts. Figure 5.1-1 illustrates the stratigraphy at R-44. A detailed lithologic log based on analysis of drill cuttings is presented in Appendix A.

Quaternary Alluvium, Qal (0-47 ft bgs)

Quaternary alluvium, consisting of unconsolidated tuffaceous silty sand to sandy silt with pebble gravels containing pumice and volcanic detritus, occurs from 0 to 47 ft bgs. No evidence of alluvial groundwater was observed.

Unit 1g of the Tshirege Member of the Bandelier Tuff, Qbt 1g (47-70 ft bgs)

Unit 1g of the Tshirege Member of the Bandelier Tuff was encountered from 47 to 70 ft bgs as interpreted by natural gamma geophysical log analysis. Unit 1g is a poorly welded vitric ash-flow tuff that is pumiceous, generally crystal rich and lithic-poor, with abundant vitric ash matrix. The thin Tshirege Unit 1g section preserved in R-44 contains strongly weathered pumices, minor lithics of diverse volcanic lithologies and abundant quartz and sanidine crystals.

Cerro Toledo Interval, Qct (70-94 ft bgs)

The Cerro Toledo interval, a thin layer of poorly consolidated volcaniclastic sediments that occurs stratigraphically between the Tshirege and Otowi Members of the Bandelier Tuff, is present from 70 to 94 ft bgs based on natural gamma ray geophysical log interpretation. This unit consists of silty fine to

medium sands and gravels made up of detrital volcanic materials (dacites, obsidian, rhyodacite), generally weathered pumice fragments, and abundant quartz and sanidine crystal grains.

Otowi Member of the Bandelier Tuff, Qbo (94-296 ft bgs)

The Otowi Member of the Bandelier Tuff is present from 94 to 296 ft bgs as interpreted from natural gamma geophysical log data. The Otowi Member is a poorly welded, pumiceous, locally lithic-rich, ash-flow tuff. Abundant pumice lapilli are white to pale orange, glassy, fibrous-textured and quartz- and sanidine-phyric and are enclosed in a matrix of vitric ash. Locally abundant volcanic lithic fragments, or xenoliths (generally up to 15 mm in diameter), are commonly subangular to subrounded and of intermediate volcanic composition, predominantly gray and light pinkish gray hornblende-and biotite-phyric dacites.

Guaje Pumice Bed of the Otowi Member of the Bandelier Tuff, Qbog (296–313 ft bgs)

The Guaje Pumice Bed occurs from 296 to 313 ft bgs on the basis of natural gamma ray log interpretation. The Guaje is a pumice-rich, lithic- and crystal-poor fall deposit that contains abundant (97%–100% by volume) pristine-appearing vitric, phenocryst-poor pumice fragments and lapilli. Trace volumes of volcanic lithics, quartz and sanidine phenocrysts, and fine ash are present.

Basaltic Volcaniclastic Sediments, Unassigned (313-344 ft bgs)

A thin sedimentary layer of pinkish to orange-tan siltstone to silty fine- to medium-grained sandstone with pebble gravel was intersected from 313 to 344 ft bgs, based on natural gamma log interpretation. Locally abundant subrounded detrital clasts (up to 20 mm in diameter) consist of basalt, basaltic scoria, vitric pumice fragments, dacite, and minor quartzite. These basalt-rich sediments occur at a stratigraphic position regionally occupied by the Puye Formation but have not yet been assigned to a particular unit. Basaltic constituents in these sediments likely were derived from underlying Cerros del Rio basalt lavas.

Cerros del Rio Basalt, Tb4 (344-707 bgs)

The Cerros del Rio basalt, intersected from 344 to 707 ft bgs, is locally a sequence of basalt lava flows with interlayers of cinders and basaltic ejecta, and pumiceous and basaltic sediments, some of which suggest a possible hydromagmatic origin. The upper part of the Cerros del Rio section, from 344 to 505 ft bgs, is made up of three distinct clinopyroxene (cpx)-phyric and olivine-cpx basalt flows, each with a layer of cinders/ejecta at its base. Cuttings suggest that a basaltic tuff layer containing basalt cinders, glassy scoria, dacite, weathered pumice, minor quartzite and fragments of indurated volcaniclastic sandstone, from 505 to 535 ft bgs, may indicate a hydromagmatic event between effusive lava eruptions. A similar sequence of three olivine-bearing basalt flows, with intercalated thin sedimentary deposits containing basalt and pumice detritus, makes up the lower part of the Cerros del Rio section between 535 and 707 ft bgs.

Puye Formation, Tpf (707–1005 ft bgs)

Puye Formation volcaniclastic sediments encountered from 707 to 1005 ft bgs consist of texturally diverse, gray, grayish brown and pinkish tan, poorly sorted, fine to coarse gravels, gravelly sandstones and silty sandstones with gravel. Detrital constituents that make up these sediments are generally subangular to subrounded and represent a range of volcanic lithologies including olivine-basalt (present as detrital clasts mainly at the top of the section), abundant biotite- and hornblende-dacites (present as a major constituent in large volumes throughout the section), rhyodacite, weathered pumice, scoria and dark colored vitrophyre.

Miocene Pumiceous Sediments, Tjfp (1005-1088 ft bgs)

A section of pumice-rich volcaniclastic sediments occur from 1005 to 1088 ft bgs. These deposits are made up of fine- to coarse-grained sandstones with pebble gravels, locally with a silty matrix. White, glassy, phenocryst-poor detrital pumices generally make up a large percent (locally as much as 100% by volume) of granule and pebble-size clasts. Additional constituents include abundant subangular to subrounded dacites, lesser amounts of basalt and andesite, and locally trace occurrences of Precambrian quartzite.

Miocene Riverine Sediments, Tcar (1088–1094 ft bgs)

A brief interval of fine to coarse gravels with fine- to coarse-grained sandstones, representing axial-river deposits, was encountered from 1088 ft bgs to the total borehole TD of 1094 ft bgs. These distinctive sediments are characterized by rounded to well-rounded pebbles and coarser gravel clasts composed of diverse volcanic litholgies (i.e., dark colored fine-grained andesites, varieties of dacite and rhyolite) and locally abundant (up to 30% by volume) Precambrian granites and quartzites.

5.2 Groundwater

Possible groundwater was first encountered at approximately 739 ft bgs in the Puye Formation sediments on January 24, 2009. As discussed in section 6.1, video log interpretation and later water-level measurements suggested that this was water introduced during drilling (see Appendix B). Perched water was not present in R-44. After the well was drilled to final depth of 1094 ft bgs, the water level was measured at approximately 879.1 ft bgs in the borehole.

Groundwater-screening samples were collected during drilling, well development, and aquifer testing as discussed in section 4.2 and presented in Table 4.2-1. Groundwater chemistry and field water-quality parameters are discussed in Appendix B. Aquifer testing data and analysis are discussed in Appendix C.

6.0 BOREHOLE LOGGING

Several video logs and a limited suite of geophysical logs were collected during the R-44 drilling project using Laboratory-owned equipment. An additional suite of cased-hole geophysical logs was collected by Schlumberger Wireline Services. A summary of video and geophysical logging runs is presented in Table 6.0-1.

6.1 Video Logging

A video log was run in the uncased borehole to check for the presence of perched groundwater on November 17, 2008. Water was observed in the video log in the Puye Formation sediments at a depth of 739 ft bgs when the borehole was at 765-ft depth. However interpretation of the log indicated the actual depth of the observed water was uncertain because there was a "knot" in the wire line. The November 17, 2008, video log from the borehole is presented on a digital video disc as part of Appendix D included with this document. Table 6.0-1 provides details about the video logging run.

6.2 Geophysical Logging

A suite of Schlumberger geophysical logs was run inside the drill casing on December 9, 2008. At the time of logging, the terminations of the two casing strings in the borehole were located at the following depths: 16-in. casing at 344 ft bgs and the 12-in. casing at 1094 ft bgs. The geophysical suite included

natural gamma ray, Triple Litho-Density (TLD), Elemental Capture Sonde (ECS), and Compensated Neutron Log (CNL). Interpretation and details of the logging are presented on CD as part of Appendix E.

7.0 WELL INSTALLATION

R-44 well was installed between December 13, 2008, and January 15, 2009.

7.1 Well Design

The R-44 well was designed in accordance with the approved Drilling Work Plan. NMED approved the well design before installation. The well was designed with dual-screened intervals to monitor groundwater quality at two depths in the upper part of the regional aquifer within Puye Formation sediments.

7.2 Well Construction

The R-44 monitoring well was constructed of 5.0-in.-I.D./5.56-in.-O.D., type A304 stainless-steel beveled casing fabricated to American Society for Testing and Materials A312 standards. The two screened sections utilized 10-ft lengths of 5.0-in.-I.D. rod-based 0.020-in. wire-wrapped well screen. Welding, using compatible stainless-steel welding rods, was used to join all individual casing and screen sections. All casing and screens were steam and pressure washed on-site before installation. A 2-in. I.D. steel threaded/coupled tremie pipe string (decontaminated prior to use) was utilized for delivery of backfill and annular fill materials during well construction. The placement of annular materials typically had two components: installing materials, and retracting the drill casing and raising the tremie pipe. As each section of drill casing was cut off the string, it was picked up and laid down. During this part of the process, the well casing was hung under full tension on a wireline while the drill casing was supported by a ring and slips.

Two screened intervals were chosen for the R-44 well design, based on monitored water levels and indications of potentially productive full-saturation intervals in the Schlumberger geophysical logs. The lower nominal 10-ft long screened interval had the top of the screen set at 985.3 ft bgs, and the upper nominal 10-ft long screened interval had the top of the screen set at 895 ft bgs. A 20.8-ft stainless-steel sump was placed below the bottom of the lower well screen. Stainless-steel centralizers (four sets of four) were welded to the well casing approximately 2.1 ft above and below each screen. A Pulstar work-over rig was used for well construction activities. Figure 7.2-1 presents an as-built schematic showing construction details for the completed well.

Well construction materials were moved onto the R-44 site starting on December 11, 2008. The Pulstar rig was moved on location and decontamination of the stainless-steel well casing and screens took place the next day. Before running the well casing, 41 ft³ of 10/20 silica sand was added to the borehole as backfill bringing the borehole bottom to 1024.7 ft bgs, which is roughly 5 ft below the 12-in. casing cut.

On December 13 the well casing was installed. Each joint was welded as it went into the borehole, using careful welding techniques and covering the borehole to avoid slag falling into the annular void. After hanging the well at 1016 ft bgs the process of installing annular materials began. Additional 10/20 silica sand (5.5 ft³) was added to bring the top of the backfill to 1008.4 ft bgs. A lower bentonite seal composed of ¼-in. pellets (1.3 ft³) followed by ¾-in. chips (0.7 ft³) was placed from 999.8 to 1008.4 ft bgs. The lower screen 10/20 silica sand filter pack was then installed, and surged to promote compaction, from 980.2–1008.4 ft bgs. This was capped by a finer 20/40 silica sand transition from 976.3 to 980.2 ft bgs on December 22, 2008. All fieldwork was suspended that day due to the Laboratory holiday shut-down and R-44 well construction recommenced on January 5, 2009, after the break.

A seal separating the two screened intervals was placed from 910.2–976.3 ft bgs and consisted of $\frac{1}{4}$ -in. bentonite pellets (13.4 ft³) followed by $\frac{3}{4}$ -in. bentonite chips (45.2 ft³). The upper screen filter pack of 10/20 silica sand was then installed (and surged) from 890.3 to 910.2 ft bgs. The upper filter pack was then capped with a transition 20/40 silica sand from 887.6 to 890.3 ft bgs.

The well's upper bentonite seal ($\frac{1}{2}$ -in. chips) was installed from 342.4 to 887.6 ft bgs from January 8 to January 12, 2009. A surface seal (mix of 97–98 wt% Portland cement with 2–3 wt% bentonite) was placed above the upper bentonite seal from 3–342.4 ft bgs; this marked well construction completion on January 15, 2009 (1030 h). Table 7.2-1 details volumes of all materials used during well construction.

8.0 POSTINSTALLATION ACTIVITIES

Following installation, the well was developed and aquifer pumping tests were performed. Total groundwater purged during well development and aquifer testing was 92,929 gal. The wellhead and surface pad was constructed and a geodetic survey performed. A dedicated dual-zone sampling system will be installed after receipt from the manufacturer. Site restoration activities will be completed following the final disposition of contained drill cuttings and groundwater, per the NMED-approved waste-decision trees.

8.1 Well Development

Well development was conducted between January 15 and January 20, 2009. Initially, the screened interval was bailed and swabbed to remove formation fines in the filter pack and well sump. Bailing and swabbing continued until water clarity visibly improved. Final development was accomplished using a submersible pump. The swabbing tool was a 4.5-in.-O.D. 1-in.-thick nylon disc attached to a weighted steel rod. The swabbing tool was lowered by wireline and drawn repeatedly in both directions across each screened interval. After bailing and swabbing, a 10-hp, 4-in.-Grundfos submersible pump was installed in the well for the final stage of well development. Approximately 16,005 gal. of groundwater was purged at R-44 during well development activities.

During the pumping stage of well development, turbidity, temperature, pH, dissolved oxygen (DO), oxygen-reduction potential (ORP), and specific conductance parameters were measured. In addition, water samples for TOC analysis were collected. The required values for TOC and turbidity to determine adequate well development are less than 2.0 ppm and less than 5 nephelometric turbidity units (NTUs), respectively.

A discussion of water removed during well development, field water-quality parameters, and analytical results for samples collected during development is summarized below in section 8.1.1 and detailed in Table B.1.2-1 of Appendix B.

8.1.1 Well Development Field Parameters

Field parameters were measured at well R-44 by collecting aliquots of groundwater from the discharge pipe without the use of a flow-through cell, allowing the samples to be exposed to the atmosphere. Results are provided here and in greater detail in Appendix B. This condition probably resulted in a slight variation of field parameters during well development and during the pumping test, most notably, temperature, pH, and DO.

Measurements of pH varied from 8.22 to 8.30 in the upper screened interval and from 8.19 to 8.29 in the lower screened interval. Measurements of temperature varied from 18.3°C to 18.56°C in the upper screened interval and from 17.47°C to 18.78°C in the lower screened interval. Concentrations of DO

varied from 9.70 to 10.66 mg/L in the upper screened interval and from 11.57 to 13.72 mg/L in the lower screened interval. Uncorrected ORP measurements varied from –135.1 to–129.7 millivolts (mV) in the upper screened interval and from –130.8 to –118.9 mV in the lower screened interval. These negative, uncorrected ORP values are not reliable and representative of known relatively oxidizing conditions characteristic of the regional aquifer beneath the Pajarito Plateau. Specific conductance ranged from 142 to 148 microsiemens per centimeter (μ S/cm) in the upper screened interval and from 193 to 204 μ S/cm in the lower screened interval. Values of turbidity measured at R-44 ranged from 0.0 to 0.1 NTU for the nonfiltered groundwater samples of the upper screen and from 0.0 to 55.8 NTUs for the lower screened samples.

8.2 Aquifer Testing

Aquifer pumping tests were conducted at R-44 from February 14 to February 17, 2009. Several shortduration tests with short-duration recovery periods were performed on the upper and lower screens in the well. A 24-h test followed by a 24-h recovery period completed the testing. The same 10-hp Grundfos pump used during well development was used to perform the aquifer tests. Approximately 76,924 gal. of groundwater was purged during aquifer testing activities.

During aquifer testing, turbidity, temperature, pH, DO, ORP, and specific conductance parameters were measured. In addition, water samples for TOC analysis were collected.

A discussion of water removed during well development, field water-quality parameters, and analytical results for samples collected during development is summarized below in section 8.2.1 and detailed in Table B.1.2-1 of Appendix B. Results of the R-44 aquifer test are presented in Appendix C.

8.2.1 Aquifer Testing Field Parameters

Measurements of pH varied from 7.80 to 8.04 in the upper screened interval and 8.31 to 8.67 in the lower screened interval at R-44. Measurements of temperature varied from 14.99°C to 19.08°C in the upper screened interval and 15.14°C to 20.31°C in the lower screened interval. Concentrations of DO varied from 7.95 to 9.30 mg/L in the upper screened interval and from 8.60 to 11.14 mg/L in the lower screened interval. Uncorrected ORP measurements varied from 117.3 to 204.4 mV in the upper screened interval and from 117.3 to 195.2 mV in the lower screened interval. The uncorrected ORP measurements are in general agreement with the DO values, suggesting that relatively oxidizing conditions were established during the aquifer performance testing at well R-44. Specific conductance ranged from 60 to 140 μ S/cm in the upper screened interval and 173 to 154 μ S/cm in the lower screened interval. Values of turbidity for the nonfiltered groundwater samples ranged from 0 to 2.8 NTUs in the upper screened interval and 1.6 to 5.9 NTUs in the lower screened interval.

8.3 Dedicated Sampling System Installation

A dedicated sampling system for the R-44 well was custom-designed based on the hydrogeologic data gathered during the aquifer tests. The sampling system is on order from the manufacturer and will be installed upon delivery. The system consists of Baski Inc.-designed stainless-steel plumbing and an inflatable isolation packer. The system will implement a shrouded 4-in, Grundfos submersible pump (environmentally retrofitted with Teflon) with a 4-in., 3-phase, 460-V, viton-fitted Franklin Electric submersible motor. The pump will draw water from discrete intervals via pneumatically actuated access port valves. An inflatable viton-covered packer will be supplied as a component of the dedicated system.

All materials that contact the groundwater will be constructed of stainless steel, Teflon, viton, or polyvinyl chloride (PVC). All components of the pump column will be new. The pump column will be constructed of 1-in. threaded/coupled stainless steel pipe with check valves installed in the pipe string every 200 ft. A weep hole will be installed at the bottom of the uppermost pipe joint to protect the pump column from freezing. To measure water levels in the well, two 1-in. I.D. schedule 80 PVC pipes will be installed to the top of the pump shroud in order to set dedicated transducers below the measured static water levels. The upper PVC transducer tube will be equipped with a 6-in. section of 0.010-in slot screen with a threaded end cap at the bottom of the tube. The lower PVC transducer tube will be equipped with a flexible nylon tube that will extend from a threaded end cap at the bottom of the PVC tube through the isolation packer to measure water levels in the lower screen interval. A weather-resistant pump control box will be installed next to the wellhead.

Post-installation construction and sampling system component installation details for R-44 are presented in Figure 8.3-2a. Figure 8.3-2b presents technical notes.

8.4 Wellhead Completion

A reinforced concrete surface pad, 10 ft \times 10 ft \times 6 in. thick, was installed at the wellhead. The pad will provide long-term structural integrity for the well. A brass survey pin was embedded in the northwest corner of the pad. A 10-in.-I.D. steel protective casing with a locking lid was installed around the stainlesssteel well riser. The concrete pad was slightly elevated above the ground surface and crowned to promote runoff. Base course was graded around the edges of the pad. A total of four bollards, painted yellow for visibility, are set at the outside edges of the pad to protect the well from traffic. All of the four bollards are designed for easy removal to allow access to the well. Details of the wellhead completion are presented in Figure 8.3-1a.

8.5 Geodetic Survey

A New Mexico licensed professional land surveyor conducted a geodetic survey on February 10, 2009 (Table 8.5-1). The survey data collected conforms to Laboratory Information Architecture project standards IA-CB02, "GIS Horizontal Spatial Reference System," and IA-D802, "Geospatial Positioning Accuracy Standard for A/E/C and Facility Management." All coordinates are expressed as New Mexico State Plane Coordinate System Central Zone (NAD 83); elevation is expressed in feet above mean sea level (amsl) using the National Geodetic Vertical Datum of 1929. Survey points include ground-surface elevation near the concrete pad, the top of the brass pin in the concrete pad, the top of the well casing, and the top of the protective casing.

8.6 Waste Management and Site Restoration

Waste generated from the R-44 project includes drilling fluids, purged groundwater, decontamination water, drill cuttings, and contact waste. A summary of the waste characterization samples collected from the R-44 well is presented in Table 8.6-1.

All waste streams produced during drilling and development activities were sampled in accordance with "Waste Characterization Strategy Form for the R-38, R-41, R-44, R-45, and R-46 Regional Groundwater Well Installation and Corehole Drilling" (LANL 2008, 103916).

Fluids produced during drilling and well development are expected to be land-applied after a review of associated analytical results per the waste characterization strategy form (WCSF) and the EP-Directorate Standard Operating Procedure (SOP) 010.0, Land Application of Groundwater. If it is determined that drilling fluids are nonhazardous but cannot meet the criterion for land application, the drilling fluids will be

evaluated for treatment and disposal at one of the Laboratory's six wastewater treatment facilities. If analytical data indicate that the drilling fluids are hazardous/nonradioactive or mixed low-level waste, the drilling fluids will be disposed of at an authorized facility.

Cuttings produced during drilling are anticipated to be land-applied after a review of associated analytical results per the WCSF and ENV-RCRA SOP-011.0, Land Application of Drill Cuttings. If the drill cuttings do not meet the criterion for land application, they will be disposed of at an authorized facility. Decontamination fluid used for cleaning the drill rig and equipment is containerized. The fluid waste was sampled and will be disposed of at an authorized facility. Characterization of contact waste will be based upon acceptable knowledge, pending analyses of the waste samples collected from the drill cuttings, purge water, and decontamination fluid.

Site restoration activities will include removing drilling fluids and cuttings from the pit and managing the fluids and cuttings in accordance with SOP-010.06, removing the polyethylene liner, removing the containment area berms, and backfilling and regrading the containment area, as appropriate.

9.0 DEVIATIONS FROM PLANNED ACTIVITIES

Drilling, sampling, and well construction at R-44 were performed as specified in "Final Drilling Plan for Regional Aquifer Wells R-44 and R-45" (TerranearPMC 2008, 105083).

10.0 ACKNOWLEDGMENTS

Patrick Longmire wrote Appendix B, Groundwater Analytical Results.

Boart Longyear drilled the R-44 borehole and installed the well.

Los Alamos National Laboratory personnel ran downhole video equipment.

Schlumberger Wireline Services performed the final geophysical logging of the borehole.

Right Bit Services and Equipment Repair welded the stainless well screen and casing.

TerranearPMC provided oversight on all preparatory and field-related activities.

11.0 REFERENCES

The following list includes all documents cited in this report. Parenthetical information following each reference provides the author(s), publication date, and ER ID. This information is also included in text citations. ER IDs are assigned by the Environmental Programs Directorate's Records Processing Facility (RPF) and are used to locate the document at the RPF and, where applicable, in the master reference set.

Copies of the master reference set are maintained at the NMED Hazardous Waste Bureau and the Directorate. The set was developed to ensure that the administrative authority has all material needed to review this document, and it is updated with every document submitted to the administrative authority. Documents previously submitted to the administrative authority are not included.

LANL (Los Alamos National Laboratory), March 2006. "Storm Water Pollution Prevention Plan for SWMUs and AOCs (Sites) and Storm Water Monitoring Plan," Los Alamos National Laboratory document LA-UR-06-1840, Los Alamos, New Mexico. (LANL 2006, 092600)

- LANL (Los Alamos National Laboratory), October 4, 2007. "Integrated Work Document for Regional and Intermediate Aquifer Well Drilling (Mobilization, Site Preparation and Setup Stages)," Los Alamos National Laboratory, Los Alamos, New Mexico. (LANL 2007, 100972)
- LANL (Los Alamos National Laboratory), November 2007. "Work Plan for Geochemical Characterization and Drilling for Fate and Transport of Contaminants Originating in Sandia Canyon," Los Alamos National Laboratory document LA-UR-07-7579, Los Alamos, New Mexico. (LANL 2007, 099607)
- LANL (Los Alamos National Laboratory), October 2008. Waste Characterization Strategy Form for the R-38, R-41, R-44, R-45, and R-46 Regional Groundwater Well Installation and Corehole Drilling, Los Alamos, New Mexico. (LANL 2008, 103916)
- TerranearPMC, October 2008. "Final Drilling Plan for Regional Aquifer Wells R-44 and R-45," plan prepared for Los Alamos National Laboratory, Los Alamos, New Mexico. (TerranearPMC 2008, 105083)

Map Data Sources for R-42 Completion Report Location Map

Point Feature Locations of the Environmental Restoration Project Database; Los Alamos National Laboratory, Waste and Environmental Services Division, EP2008-0109; February 28, 2008.

Hypsography, 100 and 20 Foot Contour Interval; Los Alamos National Laboratory, ENV Environmental Remediation and Surveillance Program; 1991.

Surface Drainages, 1991; Los Alamos National Laboratory, ENV Environmental Remediation and Surveillance Program, ER2002-0591; 1:24,000 Scale Data; Unknown publication date.

Paved Road Arcs; Los Alamos National Laboratory, KSL Site Support Services, Planning, Locating and Mapping Section; 06 January 2004; as published January 4, 2008.

Dirt Road Arcs; Los Alamos National Laboratory, KSL Site Support Services, Planning, Locating and Mapping Section; 06 January 2004; as published January 4, 2008.

Structures; Los Alamos National Laboratory, KSL Site Support Services, Planning, Locating and Mapping Section; 06 January 2004; as published January 4, 2008.

Technical Area Boundaries; Los Alamos National Laboratory, Site Planning & Project Initiation Group, Infrastructure Planning Division; September 19, 2007.





May 2009

Figure 1.0-1 Regional aquifer well R-44 with respect to surrounding regional wells and PM-5



TOTAL LENGTH OF CASING AND SCREEN (FT) 1019.0		
DEPTH TO WATER FOLLOWING 879.1 DRILLING (FT BGS)		
DIAMETER OF BOREHOLE	T	
16.75" FROM 0.0 FTTO 345.8 FT BGS 15.0" FROM 345.8 FTTO 765.0 FT BGS 12.75" FROM 765.0 FTTO 1094.0 FT BGS		
SURFACE COMPLETION INFORMATIO TYPE OF PROTECTIVE CASING SIEL SIZE <u>10"</u>	N	
SURFACE SEAL AND PAD COMPLETIO	IN IH FT (H)	2000 200
SURFACE SEAL <u>3.0</u> TO <u>342.</u>	4 (FT BGS)	0000
HYDRATED BENTONITE CHIP <u>342.4</u> TO <u>883</u> SEAL	7 <u>.6</u> (FT BGS)	20.85 0 0 0 2 3
STAINLESS-STEEL CENTRALIZERS US 2.1 FT ABOVE AND BELOW SCREENS FINE SAND COLLAR <u>887.6</u> TO <u>890</u>	ED 0.3 (FT BGS)	100000 +
SCREENED INTERVAL 895.0 TO 90	5.0 (FT BGS)	
FILTER PACK 890.3 TO 910	0.2 (FT BGS)	
BENTONITE SEAL 910.2 TO 976	6.3 (FT BGS) —	
FINE SAND COLLAR 976.3 TO 980	0.2 (FT BGS) —	2 2 2
SCREENED INTERVAL 985 3 TO 99	5.2 (FT BGS)	
FILTER PACK 980.2 TO 999	9.8 (FT BGS)	
BENTONITE SEAL 9999.8 TO 100	08.4 (FT BGS)	
BOTTOM OF CASING 1016.0 (FT	BGS)	800
BACKFILL MATERIAL 1008.4 TO 108	84.0 (FT BGS)	- ≻
SLOUGH 1084,0 TO 109	94.0 (FT BGS)	
BOTTOM OF BORING 1094.0 (FT	BGS)	200
WELL DEVELOPMENT INFORMATION: WELL COMPLETION BEGAN WELL DI DATE 12/13/08 TIME 0947 DATE 01/ WELL COMPLETION FINISHED WELL DI DATE 01/15/09 TIME 1030 DATE 01	EVELOPMENT /15/09 TIME 0930 EVELOPMENT FINIS /20/09 TIME 1315	DEVELO SHED DEVEOPM TOTAL PUI
TerranearPM	МС	
Drafted By: TPMC Date: March 23, Project Number:88100 Filerame: R44_A	2009 sBuiltWellConstruction_	Fig7-2-1_r1

Figure 7.2-1 R-44 as-built well construction diagram

		LOCKING COVER				
		ELEVATION OF WELL CAP (FT AMSL) 6717.56 ELEVATION OF PROTECTIVE CASING (FT AMS GROUND SURFACE ELEVATION (FT AMSL) 66 MONI MENT MARKEP ELEVATION (FT AMSL)	L) 6718.40 /15.10 6714.91			
		SLOPED CONCRETE PAD/SLIPEACE				
		Stored concherent	SEAL			
			2839 M (P 1988)			
		GROUT FORMULA (PROPORTION OF E CEMENT 97-98 Wt% BENTONITE 2-3 Wt%	ACH)			
		QUANTITY USED				
		CALCULATED VOLUME 405.11C				
		> 16" CASING/SHOE				
	1000	344.0-345.8 FT BGS				
	A 10	TYPE OF CASING				
	1000	STAINLESS STEEL, FLUSH WELDED CASING DIAMETER				
	202					
	0000					
	290	HYDRATED BENTONITE CHIP SEAL				
	200	CHIPS 540.7.63				
	Co.	QUANTITY USED548.7 IT				
	000	CALCULATED VOLUME				
	100	FINE SAND COLLAR				
	399	/ SIZE / TYPE 20/40 SILICA SAND				
	0 D	OUANTITY USED 3.5 ft ³				
	000	CALCULATED VOLUME 2.0 ft ³				
	12	FILTER PACK				
		QUANTITY USED _28.0 ft ³ _ CALCULATED VOLUN	14.1 ft ³			
	1	TYPE OF SCREENS STAINLESS STEEL				
		INSIDE 5.0" SLOT SIZE 0.020				
_	12	OUTSIDE 5 7/8" JOINT TYPE FLUSH WELDED				
	100		3.6 ft ³			
	000	PELLETS/CHIPS CALCULATED VOLUME 4	7.0 ft ³			
_	1.1.1.	QUANTITY USED 1.5 ft ³ CALCULATED VOLUM	2.8 ft ³			
		FILTER PACK SAND SIZE 10/20 SILICA SAND				
	1.0	QUANTITY USED 22.0 ft ³ CALCULATED VOLUME	13.9 ft ³			
-	100	BENTONITE SEAL OUANTITY USED	2.0 ft ³			
	000	PELLETS/ CHIPS CALCULATED VOLUME	6.1 ft ³			
1		BACKFILL MATERIAL QUANTITY USED	46.5 ft ³			
-	00					
-	a.C	1020.0-1094.0 FT BGS				
eve (SW/ Evec DTAL	LOPMEN ABBING D PMENT PURGE VC	FINAL PARAMETERS PH 8.22/8.19 ZBAILING ZPUMPING URGE VOLUME (GAL) 16005 SPECIFIC CONDUCTANCE (µ DLUME (GAL) 92929	(<u>18.78</u> 15/cm) <u>142/193</u>			
T	R-44	AS-BUILT WELL CONSTRUCTION DIAGRAM	Fig			
	0.004040	Mortandad Canyon	rig.			
-		Los Alamos National Laboratory	7.2-1			
		Los Alamos, New Mexico	NOT TO SCALE			



Figure 8.3-1a As-built schematic for regional well R-44

R-44 TECHNICAL NOTES: 1

SURVEY INFORMATION²

Brass Marker Northing: 1767109.8527 ft Easting: 1640061.3389 ft 6714.91 ft AMSL Elevation:

Well Casing (top of stainless steel) Northing: 1767104.3569 ft Easting: 1640063.4865 ft Elevation: 6717.56 ft AMSL

BOREHOLE GEOPHYSICAL LOGS

LANL: natural gamma ray, induction, video Schlumberger: natural gamma ray, elemental capture (ECS), compensated neutron (CNTG), litho-density (TLD)

DRILLING INFORMATION Drilling Company Boart Longyear

Drill Rig

Foremost DR-24HD

Drilling Methods

Dual Rotary Fluid-assisted air rotary, Foam-assisted air rotary

Drilling Fluids Air, potable water, AQF-2 Foam

MILESTONE DATES Drilling

11/10/2008 12/08/2008

Well Completion 12/13/2008

Start: Finished:

Start:

Finished:

Well Development Start: 01/15/2009

Finished: 01/20/2009

WELL DEVELOPMENT **Development Methods**

Performed swabbing, bailing, and pumping Total Volume Purged: 16005 gallons (both screens)

01/15/2009

Parameter Measurments (Final, upper screen/lower screen) 8.22/8.19

pH: Temperature: Specific Conductance:

18.48/18.78°C 142/193 µS/cm 0.0/0.0 NTU

NOTES:

Turbidity:

Additional information available in "Final Completion Report, Characterization Well R44 and R45, Los Alamos National Laboratory, Los Alamos, New Mexico, TBD 2009".
 Coordinates based on New Mexico State Plane Grid Coordinates, Central Zone (NAD83); Elevation expressed in feet above mean sea level using the National Geodetic Vertical Datum of 1929.

TerranearPMC		R-44 TECHNICAL NOTES Mortandad Canyon	Figure 8.3-1b	
Drafted By: TPMC Date: March 23,2009 Project Number: 86000 Filename: R44_TechnicalNotes_Fig8-3-1b_r1		Los Alamos National Laboratory Los Alamos, New Mexico		

Figure 8.3-1b As-built technical notes for R-44

EP2009-0254

AQUIFER TESTING

Performed on:

Lower Screen

Performed on:

Step-Tests and Constant Rate Pumping Tests **Upper Screen** Water Produced: 38223 gallons

24.1 gpm 02/14–17/2009 Average Flow Rate: Water Produced: 38701 gallons Average Flow Rate:

23.9 gpm 02/19–22/2009

DEDICATED SAMPLING SYSTEM

Pump Type:TBD Model: TBD TBD U.S. gpm, intake at TBD ft bgs Environmental Retrofit

Motor Type:TBD

Model: TBD

Pump Column TBD

Transducer Tubes

TRD

Transducers Type:TBD Model:TBD S/N:TBD

Date	Water (gal.)	Cumulative Water	AOF-2 Foam (gal)	Cumulative AQF-2 Foam			
Drilling	Drilling						
11/10/08	1700	1700	3	3			
11/11/08	1400	3100	5	8			
11/14/08	4500	7600	45	53			
11/15/08	6500	14,100	40	93			
11/16/08	4000	18,100	0	93			
12/02/08	1100	19,200	8	101			
12/05/08	200	19,400	0	101			
12/06/08	1300	20,700	0	101			
12/07/08	900	21,600	0	101			
Well Construction	ו		•				
12/18/08	200	21,800	n/a*	n/a			
12/19/08	7000	28,800	n/a	n/a			
12/20/08	5700	34,500	n/a	n/a			
12/21/08	1000	35,500	n/a	n/a			
12/22/08	1000	36,500	n/a	n/a			
01/05/09	1500	38,000	n/a	n/a			
01/06/09	9800	47,800	n/a	n/a			
01/07/09	5000	52,800	n/a	n/a			
01/08/09	8700	61,500	n/a	n/a			
01/09/09	3500	65,000	n/a	n/a			
01/10/09	5500	70,500	n/a	n/a			
01/11/09	2500	73,000	n/a	n/a			
01/12/09	1600	74,600	n/a	n/a			
01/13/09	2150	76,750	n/a	n/a			
01/15/09	70	76,820	n/a	n/a			
Total Volume (gal.))						
R-44		76,820					

 Table 3.1-1

 Fluid Quantities Used during Drilling and Well Construction

Note. Cumulative returns in the pit following drilling and well development are estimated to be approximately 30,000 gal.

*n/a = Not applicable. Foam use and pit use discontinued after drilling activities; therefore, no additional fluids were produced.

Table 4.2-1
Summary of Groundwater-Screening Samples Collected during
Drilling, Well Development, and Aquifer Testing of Well R-44

Location ID	Sample ID	Date Collected	Collection Depth (ft bas)	Sample Type	Analysis
Drilling	Drilling				
R-44	GW44-09-1292	11/17/08	739.0–739.5	Possible intermediate groundwater	Anions, metals
R-44	GW44-09-1315	11/17/08	739.0–739.5	Possible intermediate groundwater	Tritium
R-44	GW44-09-1293	12/07/08	920	Regional groundwater	Anions, metals
R-44	GW44-09-1294	12/07/08	957	Regional groundwater	Anions, metals
R-44	GW44-09-1295	12/07/08	977	Regional groundwater	Anions, metals
R-44	GW44-09-1296	12/07/08	997	Regional groundwater	Anions, metals
R-44	GW44-09-1297	12/07/08	1017	Regional groundwater	Anions, metals
R-44	GW44-09-1298	12/08/08	1037	Regional groundwater	Anions, metals
R-44	GW44-09-1299	12/08/08	1056	Regional groundwater	Anions, metals
R-44	GW44-09-1301	12/08/08	1076	Regional groundwater	Anions, metals
R-44	GW44-09-1300	12/08/08	1094	Regional groundwater	Anions, metals
Well Deve	lopment				
R-44	GW44-09-1272	01/18/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1273	01/18/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1274	01/20/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC
R-44	GW44-09-1275	01/20/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC
Aquifer P	ump Test				
R-44	GW44-09-1276	02/16/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1277	02/16/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1278	02/16/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1279	02/17/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1280	02/17/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1281	02/17/09	895–905	Regional groundwater, upper screen	Anions, metals, TOC
R-44	GW44-09-1282	02/21/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC
R-44	GW44-09-1283	02/21/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC
R-44	GW44-09-1284	02/21/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC
R-44	GW44-09-1285	02/21/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC
R-44	GW44-09-1286	02/22/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC
R-44	GW44-09-1287	02/21/09	985.3–995.2	Regional groundwater, lower screen	Anions, metals, TOC

Note: Tritium was submitted for off-site analysis.

Date	Depth (ft bgs)	Description	
11/17/08	0-<762.0	Run LANL natural gamma-ray, induction, and video tools. Video shows water in borehole at 739 ft bgs. Depths suspect because of "knots" in logging wire line.	
12/09/08	0–1094.0	Run Schlumberger suite: cased-hole logs consist of a natural gamma ray, ECS, CNL, and TLD after reaching TD.	

Table 6.0-1R-44 Video and Geophysical Logging Runs

Table 7.2-1 R-44 Annular Fill Materials

Material	Volume
Surface seal: cement slurry	701.4 ft ³
Upper seal: bentonite chips	548.7 ft ³
Upper fine sand collar: 20/40 silica sand	3.5 ft ³
Upper filter pack: 10/20 silica sand	28.0 ft ³
Middle seal: bentonite pellets/chips	58.6 (13.4/45.2) ft ³
Lower fine sand collar: 20/40 silica sand	1.5 ft ³
Lower filter pack:	22.0 ft ³
Lower seal: bentonite pellets/chips	2.0 (1.3/0.7) ft ³
Backfill material: 10/20 silica sand	46.5 ft ³
Backfill material: formation slough	8.9 ft ³
Potable water used in the regional aquifer (drilling and well construction)	76,820 gal.

North	East	Elevation	Identification
1767109.85	1640061.34	6714.91	R-44 brass pin embedded in pad
1767105.66	1640062.81	6715.10	R-44 ground surface near pad
1767104.88	1640063.73	6718.40	R-44 top of 10-in. protective casing
1767104.36	1640063.49	6717.56	R-44 top of stainless-steel well casing

Table 8.5-1 R-44 Survey Coordinates

Notes: All coordinates are expressed as New Mexico State Plane Coordinate System Central Zone (NAD 83). Elevation is expressed in feet above mean sea level using the National Geodetic Vertical Datum of 1929.

	-			
Location ID	Sample ID	Date Collected	Description	Sample Type
R-44	RC05-09-1519	1/26/09	Decontamination water	Liquid
R-44	RC05-09-1520	1/26/09	Decontamination water	Liquid
R-44	RC05-09-1521	1/26/09	Decontamination water	Liquid
R-44	RC05-09-1522	1/26/09	Trip blank	Liquid
R-44	RC05-09-1527	1/26/09	Drilling fluid	Liquid
R-44	RC05-09-1528	1/26/09	Drilling fluid	Liquid
R-44	RC05-09-1529	1/26/09	Drilling fluid	Liquid
R-44	RC05-09-1530	1/26/09	Trip blank	Liquid
R-44	RC05-09-1535	1/26/09	Purge water	Liquid
R-44	RC05-09-1536	1/26/09	Purge water	Liquid
R-44	RC05-09-1537	1/26/09	Purge water	Liquid
R-44	RC05-09-1538	1/26/09	Trip blank	Liquid
R-44	RC05-09-1543	1/26/09	Drill cuttings	Solid
R-44	RC05-09-1544	1/26/09	QC sample of -1543	Solid

Table 8.6-1 Summary of Waste Samples Collected during Drilling and Development of R-44

Appendix A

Well R-44 Lithologic Log
Los Alamos National Laboratory Regional Hydrogeologic Characterization Project Borehole Lithologic Log

COREHOLE IDENTIFICATION (ID) : R-44		TECHNICAL A	AREA (TA): 5	PAGE: 1 of 16	
DRILLING COMPANY: Boart Longyear Company		START DATE/ 11/10/2008:014	/TIME: 45	END DAT	TE/TIME: 12/8/2008: 1735
DRILLING	METHOD: Dual Rotary	MACHINE: For	remost DR24 HD	SAMPLIN	IG METHOD: Grab
GROUND	ELEVATION:			TOTAL D ground su	DEPTH: 1094 ft below urface (bgs)
DRILLERS	: J. Staloch, C. Johnson		SITE GEOLOGISTS:	A. Miller, C	. Pigman, J. R. Lawrence
Depth (ft bgs)		Lithology		Lithologic Symbol	Notes
0–47	ALLUVIUM: Unconsolidated tuffaceous sediments—light grayish tan (7.5YR 7/1) to pale pinkish tan (7.5YR 8/4) silty fine to medium sand with minor pebble gravel; detrital grains/clasts of indurated tuff, quartz and sanidine crystals, pumice and volcanic lithics. 0–5 ft surficial construction fill. 5–20 ft +10F: quartz and sanidine crystal grains, mixed angular volcanic clasts and fragments of siltstone/very fine-grained sandstones. 20–47 ft +10F/+35F: siltstone fragments, quartz and sanidine grains, granules of weathered pumice. 95–108 ft +10F: mixed weathered pumice fragments, subangular granules of various volcanic rocks, fragments of silty		Qal	Note: Drill cuttings for microscopic and descriptive analysis were collected at 5-ft intervals from 0 ft bgs to borehole total depth (TD) at 1094 ft bgs. Quaternary alluvial sediments, from 0 to 47 ft bgs, are estimated to be 47 ft thick. The Qal–Qbt 1g contact is estimated to be at 47 ft bgs.	
47–70	 UNIT 1g OF THE TSHIREGE MEMBER OF THE BANDELIER TUFF: Tuff—light grayish tan (7.5YR 7/1), very poorly welded, locally abundant ash matrix. 47—55 ft WR: silty ash matrix. +10F/+35F: 30%–40% fragments tuffaceous fragments with abundant quartz and sanidine grains; 30%–40% subangular lithics (up to 10 mm in diameter) of diverse intermediate volcanic rocks; 10%–20% weathered pumices. 55–60 ft +10F: 60%–70% white and gray subangular dacitic lithics; 20%–30% weathered pumices fragments; 10% fragments of silty sandstone. 60–70 ft +10F: 90-95% subrounded granule-size lithics of various volcanic lithologies (dacites, obsidian, rhyodacite(?); 10% weathered pumices fragments. 		Qbt 1g	Unit 1g of the Tshirege Member of the Bandelier Tuff, from 47 to 70 ft bgs, is estimated to be 23 ft thick. The Qbt 1g–Qct contact is estimated to be at 70 ft bgs, based on natural gamma log interpretation.	

Borehole ID: R-44		TA: 5	Page: 2	of 16	
Depth (ft bgs)		Lithology	Lithologlc Symbol	Notes	
	CERRO TOLEDO INTERV	AL OF THE BANDELIER TUFF:		The Cerro Toledo	
	Tuffaceous sediments—lig pinkish tan (7.5YR 8/4) poo silty fine- to medium-graine locally abundant ash matrix	ht grayish tan (7.5YR 7/1) to pale orly consolidated to unconsolidated ed sandstone with local pebble gravel, k.		interval, from 70 to 94 ft bgs, is estimated to be 24 ft thick. The Qct–Qbo contact is	
70–94	70–80 ft +10F/+35F: 30%- sandstone with abundant of 30%–40% subangular lithic intermediate volcanic lithol	40% fragments of tuffaceous silty uartz and sanidine grains; cs (up to 10 mm in diameter) of diverse ogies; 10%–20% weathered pumices.	Qbo	estimated to be at 94 ft bgs, based on natural gamma log interpretation.	
	80–94 ft+10F: very little ma minor dacite granules. +35 crystal grains, pumices and	Iterial retained of this size fraction; F: abundant quartz and sanidine d volcanic lithic fragments.			
	OTOWI MEMBER OF THE BANDELIER TUFF:		Qbo	Otowi Member ash-flow	
	Tuff—pale orange tan (7.5YR 8/6) to pinkish white (7.5YR 8/2), poorly welded, lithic-bearing, locally abundant ash and silt matrix; this interval likely represents the weathered upper part of Qbo.			to 296 ft bgs, is estimated to be 202 ft	
94–110	94–105 ft +10F: mixed weathered pumice fragments, various subangular volcanic lithics.			unck.	
	105–110 ftWR: abundant silt and ash matrix. +10F: 75% weathered to glassy pumices (up to 20 mm in diameter); 15%–25% dacite fragments.				
	Tuff—white (7.5YR 8/2), po bearing, abundant ash mat	oorly welded, crystal-rich, lithic- rix.			
110–120	110—120 ft +10F: 100% p diameter), mostly glassy to sanidine-phyric. +35F: 30% 40%–45% pumice fragmer	umice fragments (up to 22 mm in locally devitrified, quartz- and 6–35% quartz and sanidine crystals, hts (mostly glassy).	Qbo		
120–135	Tuff—white (10YR 8/1), po lithic-bearing, abundant as 120–135 ft +10F: 80%–859 14 mm in diameter), quartz 15%–20% broken volcanic	orly welded, pumice-rich, crystal-rich, h matrix. % vitric pumice fragments (up to - and sanidine-phyric. +35F: lithic fragments (up to 15 mm)	Qbo	125—135 ft +10F contains large dacite fragments up to 25 mm in diameter.	
	30%–40% quartz and sanio pumice fragments; 15%–20	dine crystals, 40%–50% glassy % lithic fragments.			
	Tuff—white (10YR 8/1), po lithic-poor, abundant fine v	orly welded, pumice-rich, crystal-rich, olcanic ash matrix.			
135–145	135—145 ft +10F: 100% vi diameter), quartz- and sani +35F: 40%–50% quartz an 10%–15% volcanic lithic gr	tric pumice fragments (up to 20 mm in dine-phyric; trace volcanic lithics. d sanidine crystals, 30%–40% ains.	Qbo		

Borehole	ID: R-44	TA: 5	Page: 3	of 16
Depth (ft bgs)		Lithology	Lithologic Symbol	Notes
145–155	Tuff—white (10YR 8/1), pc lithic-bearing to lithic-rich, r 145—155 ft +10F: 40%–50 pumice fragments; 40%–50 gray and pinkish dacites. + crystals, 30%–40% glassy lithic fragments. 125–135 ft 10%–20% light up to 25 mm in diameter.	oorly welded, pumiceous, crystal-rich, moderate volcanic ash matrix. 0% white vitric quartz-sanidine-phyric 0% broken, angular and subangular 35F: 50%–60% quartz and sanidine pumice fragments, 10%–15% volcanic gray dacite fragments (i.e., xenoliths)	Qbo	
155–170	Tuff—pale yellowish tan (1 lithic-rich, crystal-rich, abu 155—160 ft +10F: 40%–50 intermediate volcanic com common); 40%–50% white sanidine-phyric. 160–170 ft contains 65%– dacite lithic fragments (up	0YR 7/4), poorly welded, pumiceous, ndant ash matrix. 0% angular lithics (up to 9 mm) of positions (porphyritic dacites are a to pink glassy pumices, quartz- 70% angular, porphyritic biotite-phyric to 18 mm in diameter).	Qbo	
170–175	Tuff—white (10YR 8/1), poorly welded, pumiceous, lithic-bearing, crystal-rich, abundant volcanic ash matrix. 170—175 ft +10F: 100% white glassy pumices, quartz- and sanidine-phyric. +35F: 20%–30% glassy pumice fragments, 30%–40% quartz and sanidine crystals, 30%–40% dacitic grains.		Unit 1g, Qbt	
175-185	Tuffaceous/volcaniclastic s unusual to find tuffaceous ash-flow tuffs.) —pale pink 8/1) poorly consolidated, s gravels. 175–180 ft+10F: pale pink 40%–50% broken/angular 12 mm in diameter) comport rhyolite; 40%–50% vitric pus anidine-phyric. 180-185 ft +10F: white (10 volcanic lithic fragments/cl diverse composition: pink a rhyolites, dark brown ande 5%–10% vitric white and p phyric.	sediments (?? Tuff ?? It would be sediments in the middle of the Qbo ish tan (2.5YR 8/4) to white (10YR ilty fine to medium sand and pebble ish tan (2.5YR 8/4), volcanic lithic fragments/clasts (up to osed of dacite and flow-banded umices grains, quartz- and YR 8/1), 90%–95% broken/angular asts (up to 11 mm in diameter) of and gray dacites, flow-banded site(?), minor vesicular basalt; inkish pumices, quartz- and sanidine-	Qbo	

Borehole	ID: R-44	TA: 5	Page: 4	of 16	
Depth (ft bgs)		Lithology	Lithologic Symbol	Notes	
185-220	Tuff—pale orange tan (5YF lithic-poor, crystal-rich. 185—195 ft WR/+10F: 979 limonite-stained) vitric quar (up to 18 mm in diameter), secondary Fe-oxide; 1%–3 20%–30% glassy pumice f sanidine crystals, 20%–30% 195'–200 ft WR: more abur +10F: contains 25%–35% 200–215 ft+10F: similar to 215–220 ft +10F: similar to 215–220 ft +10F: s5-90% of sanidine-phyric pumice frag 10%–15% volcanic lithic frag	 R 7/4), poorly welded, pumiceous, 6–99% white to pale orange (i.e., tz-sanidine-phyric pumice fragments commonly with black specks of % dacite lithic fragments. +35F: ragments, 50%–60% quartz and % volcanic lithic grains. ndant pale orange volcanic ash matrix. biotite-phyric dacite lithics. 185—190 ft. white to pale orange vitric quartz-gments (up to 13 mm in diameter); agments. 	Qbo		
220–240	Tuff—pale orange tan (7.5 lithic-rich, crystal-rich, loca 220–230 ft WR: abundant +10F: 75%–80% white to p phyric pumice fragments (u exhibiting black specks of s to light gray biotite-dacite li 40%–50% glassy pumice f sanidine crystals, 5%–10% 230—235 ft +10F: 60%–70 pumice, quartz- and sanidi dark Fe-oxide specks; 30% to 8 mm in diameter) comp pink and gray dacites, bank vitrophyre. 235—240 ft +10F: similar t	YR 7/6), poorly welded, pumiceous, lly abundant ash matrix. pale orange volcanic ash matrix. pale orange vitric quartz-sanidine- up to 14 mm in diameter), frequently secondary Fe-oxide; 20%–25% pinkish thic fragments. +35F: ragments, 30%–40% quartz and volcanic lithic grains. 9% white and pale orange glassy ne phyric, commonly with abundant 5–40% subangular volcanic lithics (up osed of various volcanic lithologies: ded dacites, dark porphyritic	Qbo		
240–250	Tuff—very pale orange tan pumiceous, lithic-poor, crys 240–250 ftWR: locally with matrix. +10F: 99%–100% v sanidine-phyric pumice fra locally abundant specks of lithics. +35F: 50%–60% qu 20%–30% dacite grains; 20	(7.5YR 8/4), poorly welded, stal-rich. abundant pale orange volcanic ash white to pale orange-pink vitric quartz- gments (up to 25 mm in diameter) with dark secondary Fe-oxide; <1% dacite artz and sanidine crystals; 0%–30% pumice fragments.	Qbo		

Borehole	ID: R-44	TA: 5	Page: 5	Page: 5 of 16	
Depth (ft bgs)		Lithology	Lithologic Symbol	Notes	
250–265	Tuff—pinkish white (7.5YR 8/2), poorly welded, pumiceous, lithic- bearing to lithic rich, crystal-rich. 250—265 ft +10F: 65%–75% white to pale pink glassy quartz- sanidine-phyric pumice fragments (up to 14 mm in diameter) with locally abundant specks of dark secondary Fe-oxide; 35%–25% angular fragments (up to 10 mm in diameter) composed of dark and light gray dacites. +35F: 30%–40% quartz and sanidine crystals; 25%–35% dacite lithic grains; 30%–40% pumice fragments.		Qbo		
265–285	Tuff—pale pinkish white (7 lithic-bearing, crystal-rich. 265—270 ft +10F: 95%–98 vitric, quartz-sanidine-phyr diameter); 2%–5% gray an in diameter). +35F: 50%–6 15%–25% dacitic lithic frag vitrophyre; 15%–25% pum 270–275 ft+10F: note increa lithics–80% vitric pumices; rhyolite) fragments (up to 7 275–285 ft no cuttings ava	2.5YR 8/2), poorly welded, pumice-rich, 3% white to pale pinkish white, fibrous, ic pumice fragments (up to 23 mm in id light pink dacite lithics (up to 6 mm 90% quartz and sanidine crystals; gments with minor basalt, black ice grains. eased abundances and varieties of 20% mixed volcanic (dacite, basalt, 7 mm in diameter). ilable for description.	Qbo		
285–296	Tuff—pale pinkish white (7 lithic-poor, crystal-poor. 285–296 ft WR: silty matrix quartz-sanidine-phyric pun diameter); 2%–5% dacite I of this sample size fraction 10%–15% quartz and sani fragments (dacite, black vi	2.5YR 8/2), poorly welded, pumice-rich, (x. +10F: 95%–98% white fibrous, vitric, nice fragments (up to 22 mm in ithics. +35F: Note poor representation 1, 50%–60% pumice fragments; dine crystals; 20%–25% volcanic lithic trophyre).	Qbo	The Qbo-Qbog contact is placed at 296 ft bgs, based on interpretation of natural gamma geophysical log data.	
296–313	GUAJE PUMICE BED: Tuff— white (5YR 8/1) to v rich, lithic-poor, crystal-poor 296—305 ft WR/+10F: 979 weak limonite-staining) vitr 2%–3% dacitic lithic fragm 305—315 ft WR/+10F: 100 pumices (up to 22 mm in d pristine, very fresh appear 310—315 ft +35F: no retur	ery pale orange (7.5YR 7/6), pumice- or, no apparent volcanic ash matrix. %–98% white to locally yellowish (i.e., ric pumices (up to 22 mm in diameter); ents (up to 20 mm in diameter). % white and locally pinkish vitric liameter); phenocryst-poor, having ance. ms of this sample size fraction.	Qbog	The Guaje Pumice Bed, from 296 to 313 ft bgs, is estimated to be 17 ft thick. The contact between Qbog and underlying basalt-rich sediments is placed at 313 ft bgs, based on interpretation of natural gamma geophysical log.	

Borehole	ID: R-44	TA: 5	Page: 6	of 16
Depth (ft bgs)		Lithology	Lithologic Symbol	Notes
313–340	BASALT-RICH SEDIMEN Pale orange tan (7.5YR 7/6 grained sandstone with pel 313–325' ft +10F: 60% ora siltstone with fine basalt gr 20% broken to subangular black basalt scoria. 325—330 ft +10F: 100% la subrounded clasts of black orange-tan siltstone. +35F 10% pumice fragments plu 10% basalt and dacitic gra 330–340 ft +10F: No samp	TS: 5) siltstone to silty fine- to medium- bble gravel. nge-tan fragments of indurated ains; 20% fragments of vitric pumice; clasts of hornblende-dacite and minor arge pebbles (up to 20 mm in diameter) basalt scoria with adhered rinds of 80% siltstone fragments, s quartz and sanidine crystals, ins. le recovery.	N/S	Unassigned basalt-rich volcaniclastic sediments, encountered from 313 to 344 ft bgs, are estimated to be 31 ft thick.
340–344	Pale pinkish tan (7.5YR 8/4 with subordinate chips of b 340–345 ft. Coarse- to med 340–345 ft WR: 100% silts +10F: 40% silt-coated base 60% siltstone and fine-grait dacite, and quartzite grant	 4) siltstone and fine-grained sandstone asalt-bearing siltstone similar to dium-grained sand with pebble gravel. tone fragments with basalt pebbles. alt pebbles (up to 15mm), ned sandstone fragments with basalt, les. 	N/S	Estimated contact between basalt-rich sediments and underlying Tb4 is placed at 344 ft bgs, based on natural gamma log interpretation.
344–375	CERROS DEL RIO BASA Basalt lava-medium gray (porphyritic with aphanitic g plagioclase and minor olivi 344-355 ft +10F/+35F: 100 2%-4% by volume, anhed brown to opaque cpx and r green olivine; olivine comm 355-375 ft +10F/+35F: 100 similar to 344-355 ft; degree downward.	LT: GLEY1 6/0) strongly vesicular, roundmass, clinopyroxene (cpx), ne present as phenocrysts. 0% basalt chips, phenocrysts ral (up to 3 mm in diameter) dark ninor small (up to 1 mm in diameter) nonly intergrown with cpx. 0% basalt chips, compositionally se of vesicularity diminishing rapidly	Tb4	The Cerros del Rio basalt section, encountered from 344 to 707 ft bgs, is estimated to be 363 ft thick. 344–355 ft represents the strongly vesicular top of cpx-basalt flow.
375–392	Basaltic cinder deposits-da orange brown (2.5YR 5/6) 375–395 ft WR/+10F: 95% orange-brown ferruginous 3%–5% vesicular crystal-p abundant white amygdaloid scoria.	ark reddish brown (2.5YR 4/4) to scoriaceous basalt. –97% scoriaceous basalt chips and cinders (up to 20 mm in diameter); oor cpx-basalt chips, trace locally dal zeolite (?) and zeolite-encrusted	Tb4	

Borehole	ID: R-44	TA: 5	Page: 7	of 16
Depth (ft bgs)		Lithology	Lithologic Symbol	Notes
392–435	Basalt lava–light gray (GLE phenocryst-poor, aphanitic basalt, moderately altered 392–435 ft+10F/+35F: 99% basalt, minor basalt scoria: anhedral clinopyroxene (up (up to 1 mm in diameter) g intergrown.	EY1 7/0), weakly vesicular to massive, groundmass, clinopyroxene-bearing groundmass. 6 basalt chips of altered cpx-phyric phenocrysts (2%–4% by volume) of to 2 mm in diameter) and minor small reen olivine; olivine and cpx commonly	Tb4	392–435 ft characteristic of this lava is the strong recrystallization of groundmass feldspars yielding bleached coloration and webs/tiny veinlets of clay; dusty appearance and rounding (i.e., apparent milling because of the drilling process) of chips.
435–468	Basalt lava–light gray (GLE weakly porphyritic with aph strongly altered. 435–455 ft +10F: 100% ba or milled by drilling process volume of anhedral dark bu diameter) and lesser small diameter); cpx and olivine Groundmass is distinctive and recrystallized. 455–460 ft WR/+10F: 1009 drilling process. Olivine be to 2 mm in diameter) as eu section. 460–468 ft: Similar to 435	EY1 7/1) massive to weakly vesicular, manitic GM, groundmass feldspar salt chips that are commonly rounded s, sparse phenocrysts 2%–3% by rown clinopyroxene (up to 1 mm in green translucent olivine (<1 mm in are commonly intergrown. in that the felty feldspars are bleached % basalt chips, edges milled during coming more abundant and large (up hedral phenocrysts downward in -455 ft.	Tb4	
468–471	Basaltic lava and cinder de 7/1) and reddish brown (10 GM and scoriaceous basal WR: finely milled basalt ch produced from altered feld 468–471 ft: +10F: 70% ang massive to weakly vesicula anhedral olivine (up to 3 m (1 mm in diameter) cpx; bla rinds/overgrowths of oliving chips of brick-red scoriace	eposits–varicolored light gray (GLEY1 DYR 5/6) mixed basalts with altered t. ips with abundant white powder spars. gular to rounded (milled) basalt chips, ar, phenocrysts 3%–5% by volume m in diameter) and lesser small ack cpx commonly occurs as e. GM strongly bleached; 30% angular bus basalt.	Tb4	

Borehole ID: R-44		TA: 5	Page: 8	of 16	
Depth (ft bgs)		Lithology	Lithologic Symbol	Notes	
471–490	Basalt lava–light gray (GLE groundmass; minor reddisl 471–475 ft WR/+10F: 75% olivine+cpx-basalt, ground broken chips of ferruginous 475–485 ft WR: abundant chips abraded/milled. +10F porphyritic basalt, phenocr to 2 mm in diameter) and s cpx, groundmass feldspars 3%–5% chips of hematite- 485–490 ft+10F: Locally m basalt scoria chips.	EY1 6/1) basalt chips, massive, altered n (10YR 5/6) basalt scoria. –80% abraded/milled chips mass strongly altered. 20%–25% s basalt scoria. finely ground white powder; basalt F/+35F: 95%–97% chips of olivine-cpx ysts of anhedral pale green olivine (up mall (up to 1 mm in diameter) black are bleached/recrystallized. stained basalt scoria. ore abundant (15%–20% by volume)	Tb4		
490–505	Basalt lava–light gray (GLE to weakly vesicular olivine- altered groundmass. 490–505 ft WR/+10F: 99% phenocrysts (2%–4% by vo olivine and cpx; groundma altered/recrystallized/bleac chips.	EY1 6/1) moderately altered massive cpx basalt, with moderately to slightly –100%% massive basalt chips, blume) of small (1 mm in diameter) ss feldspar hed; up to 1% reddish brown scoria	Tb4		
505–530	Basaltic cinders and volcar gray (GLEY 6/1) and brick- basaltic lava and scoria/cir 505–515 ft WR/+10F: 50% olivine-phyric basalt; groun recrystallized/bleached. 50 glassy scoriaceous cinders moderately abundant fragr 515–530 ft +10F/+35F: Mix glassy scoria, minor dacitic volcaniclastic sediments.	hiclastic sediments-varicolored light red (2.5 YR 5/8), mixed massive iders. light gray chips of cpx- and minor dmass feldspar % hematitic vesicular basalt and 6 (up to 15 mm in diameter). +35F: ments of volcaniclastic sediments. ted basalt fragments of brick-red c detritus and fragments of fine-grained	Tb4	505–530 interval possibly of hydromagmatic origin.	
530–535	Pumiceous volcaniclastic s (GLEY1 6/1), reddish brow fragments of basalt, pumic 530–535 ft WR +35F/+10F subrounded detrital grains basalt. 20%–25% fragmen phyric pumice, detrital grai	ediments-varicolored light gray n (10YR 4/8) and white (10YR 8/1) e and brown scoria. : 55%–65% chips and partly of weakly vesicular cpx-bearing ts of weathered quartz- and sanidine- ns of dacite.	Tb4	530–535 ft some evidence of reworked volcanic materials indicated by local subrounding of basalt, pumice, and dacite fragments.	

Borehole	ID: R-44	TA: 5	Page: 9	of 16
Depth (ft bgs)		Lithology	Lithologlc Symbol	Notes
535–560	Basalt lava–reddish gray (cpx-basalt, porphyritic with feldspars weakly altered at 535–540 ft WR/+10F: 1009 (3%–5% by volume) of ant in diameter); groundmass calcite on fractured surface	10YR 5/1) massive to weakly vesicular aphanitc groundmass, groundmass nd bleached. % angular basalt chips, phenocrysts nedral opaque black cpx (up to 3 mm weakly bleached; minor local white as.	Tb4	
560–575	Basalt lava–light gray (GLE cpx-basalt, porphyritic with moderately altered. 560–565 ft +10F/+35F: 95- basalt, cpx-phenocrysts (2 moderately recrystallized/b granules/grains of pumice 565–575 ft+10F/+35F: mir volcaniclastic sandstone.	EY1 7/1) massive to weakly vesicular aphanitc groundmass that is -97% angular chips of cpx-phyric %–4% by volume); groundmass bleached. 3%–5% subangular detrital and quartzite. hor to trace fragments of fine-grained	Tb4	
575–590	Basalt lava–light gray (GLE porphyritic with strongly alt 575–580 ft +10F/+35F: 80 ^o because of drilling) chips o of pumice and quartz cryst fine-grained sandstone. 580–590 ft+10F/+35F: trac	EY1 7/1) massive cpx-basalt, ered aphanitic groundmass. %–85% subrounded (i.e., milled of cpx-basalt; 15%–20% detrital grains al, also fragments of pale tan clay and the abundances of pale orange clay.	Tb4	
590–650	Basalt lava–light gray (GLE cpx- and ol-phyric basalt, p altered aphanitic groundma 590'–610 ft +10F/+35F: 10 of drilling) chips of basalt, p subhedral cpx (up to 1 mm (up to 4 mm in diameter) th cpx; groundmass altered a fragments. 610–620 ft +10F/+35F: 999 altered groundmass; <1% quartzite, and tan clay frag 620–630 ft +10F: olivine pl overgrowths. 630–650 ft +10F/35F: 98% exhibiting strongly altered a frequently have cpx overgr pumice, quartz crystal and sandstone.	EY1 7/1) massive to weakly vesicular porphyritic with moderately strongly ass. 0% subrounded (i.e., milled because obenocrysts (3%–5% by volume) of in diameter) and lesser green olivine nat are commonly rimmed by black nd bleached; trace pale tan clay %–100% cpx-basalt chips with strongly detrital grains of ferruginous scoria, ments. henocrysts frequently exhibit cpx –99% milled cpx-olivine basalt chips groundmass; olivine phenocrysts owths 1%–2% detrital grains of fragments of very fine-grained silty	Tb4	647–648 ft possible thin sedimentary interlayer containing pumice fragments and basalt granules.

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Borehole	ID: R-44	TA: 5	Page: 10	D of 16
Depth (ft bgs)		Lithology	Lithologic Symbol	Notes
650–665	Basalt lava–light gray (GLE weakly porphyritic with alter 650–665 ft WR/+10F: 1009 drilling) chips of ol-basalt, j small anhedral green olivin moderately to strongly alte fracture surfaces; trace fra	EY1 7/1) massive olivine-phyric basalt, ared aphanitic groundmass. % subrounded (i.e., milled because of ohenocrysts (1%–3% by volume) of he and trace cpx; groundmass red and bleached; minor white clay on gments of light pink claystone.	Tb4	
665–685	Basaltic volcaniclastic sedi medium gray (GLEY16/1) a detrital clasts/grains of pun 665–670 ft +10F: 100% su pebbles/clasts (up to 17 m glassy quartz- and sanidin amounts of gray massive a 670%–685 ft +10F: 40%–5 rounded) and chips; 40%– pumices and fragments of pumiceous).	ments–varicolored white (2.5YR 8/1), and reddish brown (2.5YR 4/6), mixed nice and basalt bangular to subrounded detrital m in diameter) composed mostly of e-phyric pumices with subordinate and reddish scoriaceous basalt. 10% gray basaltic detrital clasts (locally 50% pale pinkish porphyritic, vitric welded tuff (crystal-rich, lithic-bearing,	Tb4	665–685 ft apparent sedimentary interlayer between Tb4 basalt flows.
685–698	Basalt lava—medium gray phyric, phenocryst-poor, m groundmass feldspars. 685–698 ft WR/+10F: 99% phenocrysts (1%–2% by vo (up to 1mm in diameter); g recrystallized and bleached claystone.	(GLEY1 6/1) massive basalt, olivine- oderate very fine-grained alteration of –100% angular basalt chips, olume) of pale green anhedral olivine roundmass feldspars moderately d; minor fragments of white clay or	Tb4	
698–707	Basalt lava and volcaniclas gray (GLEY1 6/1) to light p chips/detritus of olivine-bas 698–707 ftWR/+10F: 80%- detrital granules of olivine- subrounded detrital volcan including gray dacite, white note trace white clay adhee	stic sediments—varicolored medium inkish tan (2.5YR 8/3), mostly salt and lesser volcaniclastic detritus. -90% angular chips and subrounded phyric basalt; 10%–20% subangular to ic clasts (up to 7 mm in diameter) e pumices, red scoriaceous cinders; red to detrital basaltic grains.	Tb4	698–707 ft apparent rubbly base of basaltic flow with intercalated thins volcaniclastic sedimentary layer. Estimated Tb4-Tpf contact placed at 707 ft bgs.
707–715	PUYE FORMATION: Volcaniclastic sediments— pale tan (5YR 8/3) coarse sand to silty sand matrix, s volcanic compositions 707–715 ft WR/+10F: 40% olivine-basalt granules (up 60% subangular to subrou 22 mm in diameter) compo	evaricolored light gray (GLEY1 7/0) and to medium gravels with fine-grained ubangular to rounded clasts of diverse subrounded light and dark gray to 10 mm in diameter); nded pebbles and broken clasts (up to used of dacites, minor pinkish pumice	Tpf	Puye volcaniclastic sediments, encountered from 707 to 1005 ft bgs, are estimated to be 298 ft thick.

and fragments of indurated dacitic silty sandstone.

Borehole	ID: R-44	TA: 5	Page: 1	1 of 16
Depth (ft bgs)		Lithology	Lithologlc Symbol	Notes
715–735	Volcaniclastic sediments— 7/2) to light gray (GLEY1 7 fine-grained sandy to silty dacite and minor basalt. 715–735 ft WR/+10F: 90% rounded clasts (up to 25 m phyric dacites; 5%–10% fr sandstone. +35F: subangu 75%–80% dacites; 20%–2 sanidine crystals; trace rec	-varicolored pale pinkish gray (5YR 7/1) coarse to medium gravels with matrix, subangular to rounded clasts of -95% broken and subrounded to im in diameter) light gray bt- and hbn- agments to indurated fine-grained ilar to subrounded grains: 5% basalt; 3%–5% quartz and d scoria.	Tpf	
735–750	Volcaniclastic sediments— with fine- to medium-grain subrounded clasts domina 10F: broken and subround mostly porphyritic dacites; minor weathered pumices.	-light gray (GLEY1 7/1) coarse gravels ed sandstone, subangular to ntly of dacites. led clasts (up to 19 mm in diameter) 10%–15% reddish vesicular basalt,	Tpf	
750–760	Volcaniclastic sediments— (5YR 7/2) medium- to coar pebble gravel, detritus con and bt-dacites. 750—760 ft WR/+10F: bro 13 mm in diameter), 70% v 30% fragments of silty fine	-light gray (GLEY1 7/1) to pinkish gray se-grained sandstone with minor sposed dominantly of porphyritic hbn- ken and subrounded clasts (up to white and grayish bt-phyric dacites -grained volcanic sandstone.	Tpf	
760–785	Volcaniclastic sediments— pinkish gray (5YR 7/2) coa grained sandstones, detrita lithologies, predominantly 760–770 ft WR/+10F: suba 17 mm in diameter) light g dark brown andesite. 770–785 ft +10F: very coa large (up to 20 mm in diam dacites, minor dark gray vi indurated sandstone.	-varicolored light gray (GLEY1 7/1) to irse gravels and medium- to coarse- al clasts composed of various volcanic dacites. angular to subrounded clasts (up to ray to pinkish porphyritic dacites, minor rse gravels indicated by abundantly neter) broken chips of porphyritic trophyre, and minor fragments of	Tpf	
785–795	Volcaniclastic sediments— coarse- to medium-grained predominantly dacitic detri 785—795 ft WR/+10F: sub 4 mm in diameter) almost	light pinkish gray (7.5YR 7/1) very d sandstones with small pebbles, tus. bangular to subrounded granules (up to exclusively of gray porphyritic dacites.	Tpf	

Borehole	ID: R-44	TA: 5	Page: 12	2 of 16
Depth (ft bgs)		Lithology	Lithologic Symbol	Notes
795–815	Volcaniclastic sediments—light pinkish gray (7.5YR 7/1) very coarse- to medium-grained sandstones with silt, composition of grains predominantly dacitic. 795—815 ft WR/+10F: subangular to subrounded grains and granules mostly of gray to pinkish gray dacites, lesser abundances other volcanics.		Tpf	795–815 ft silt percentage of silt increasing downward in this interval.
815–840	Volcaniclastic sediments— 7/0) to light pinkish gray (7 to coarse-grained sandstor dacitic. 815–820 ft WR/+10F: suba 18 mm) mostly gray porph gray porphyritic vitrophyre, 820–830' ftWR/+10F: clast dacites, white bt-phyric dar vitrophyre. 830–840 ft WR/+10F: suba 13 mm) almost exclusively dacite.	-varicolored light pinkish gray (GLEY1 55YR 7/1) coarse gravels and medium- nes; clast composition predominantly angular to subrounded clasts (up to yritic dacites, minor orange and dark trace cpx-phyric basalt. t composition more diverse: gray cite, gray and white dacitic(?) angular to subrounded clasts (up to light gray dacites, trace white bt-	Tpf	
840-860	Volcaniclastic sediments—pale pinkish tan (7.5YR 7/3) fine to coarse gravels and fine- to medium-grained sandstone with silt; clast composition predominantly dacitic. 840–850 ft WR: silt-rich matrix. +10F: broken and subangular to subrounded granules and small pebbles (up to 12 mm) mostly of light gray porphyritic dacites, minor white bt-phyric dacite. 850–860 ft texturally similar to 840–850 ft; contains also 2%–3% fragments of medium- to coarse-grained silty sandstone, also minor porphyritic vitrophyre.		Tpf	
860–870	Volcaniclastic sediments— medium gravel and mediuu predominantly dacitic detri 860–870 ft WR/+10F: brok clasts (up to 11 mm in diar light gray porphyritic dacite +10F	-pale pinkish gray (7.5YR 7/2) fine to m- to coarse-grained sandstone, tus. ten and subangular to subrounded neter) composed almost exclusively of es, minor white bt -phyric dacite matrix.	Tpf	
870–875	Volcaniclastic sediments— grained to very coarse-gra gravel, clasts predominant 870–875 ft WR: moderatel subangular to subrounded gray porphyritic dacite, wh of indurated sandstone.	-pale pinkish gray (7.5YR 7/2) fine- ined sandstone to silty sandstone with ly dacitic. y silty matrix. +10F: broken and clasts (up to 16 mm in diameter) of ite bt-bearing dacite, and minor chips	Tpf	

Borehole	ID: R-44	TA: 5	Page: 13	3 of 16
Depth (ft bgs)		Lithology	Lithologlc Symbol	Notes
875–890	Volcaniclastic sediments— coarse gravel and medium predominantly dacitic. 875–885 ft WR/+10F: brok 10 mm in diameter) of gray white bt-phyric dacite(?); m grained sandstone. 885–890 ftWR/+10F: broke 18 mm in diameter) of light hornblende (hbn)-biotite (b	pale pinkish gray (7.5YR 7/2) fine to - to coarse-grained sandstone, clasts en and subangular clasts (up to / porphyritic dacites, pink dacites, ninor fragments of indurated medium- en to subrounded detrital clasts (up to gray dacites and reddish brown t)-bearing dacites.	Tpf	
890–900	Volcaniclastic sediments— fine- to coarse-grained san gravel, clasts predominant 890–900 ftWR: silty matrix (up to 15 mm in diameter) and minor bt-bearing dacity	very pale pinkish gray (7.5YR 7/1) adstones with gravel to silty sand with ly dacitic. . +10F: broken and subangular clasts composed of porphyritic hbn-dacite e.	Tpf	
900–905	Volcaniclastic sediments— light gray (GLEY1 7/0) fine dacitic detritus. 900–905 ft +10F: broken a in diameter) of light gray hi dacite(?).	pale pinkish gray (7.5YR 7/1) to very - to coarse- grained sandstones, nd subrounded granules (up to 5 mm on-dacite and minor white bt-phyric	Tpf	
905–920	Volcaniclastic sediments— gray (7.5YR 7/1) coarse gr clasts, predominantly dacit 905–920 ft WR/+10F: brok clasts (up to 17 mm in diar porphyritic hbn-dacites, mi	light gray (GLEY1 7/0) to pale pinkish avels with medium- to coarse-grained, ic. en and subangular to subrounded neter) composed mainly of light gray nor bt-bearing dacite.	Tpf	
920–940	Volcaniclastic sediments— medium to coarse gravels sandstones, dacite-rich de 920–935 ft WR: moderatel subangular clasts (up to 10 predominantly of light gray bearing dacite. 935–940 ft WR: silty fine sa similar to 920–935 ft. +35 grained silty sandstone.	every pale pinkish gray (7.5YR 7/1) and silty medium- to coarse-grained tritus y silty matrix. +10F: broken and 0 mm in diameter) composed hbn-dacites and minor white bt- and with gravel. +10F: compositionally F: abundant fragments very fine-	Tpf	

Borehole	ID: R-44	TA: 5	Page: 14 of 16			
Depth (ft bgs)		Lithology	Lithologic Symbol	Notes		
940–960	Volcaniclastic sediments— to coarse gravels and coar detritus. 940–950 ft +10F: broken a diameter) composed predo dacites and minor bt-bearin 950–960 ft +10F: broken a diameter) of hbn- and bt-p rhyodacite(?).	-pale pinkish gray (7.5YR 7/1) medium se-grained sandstones, dacite-rich nd subangular clasts (up to 12 mm in pminantly of light gray porphyritic hbn- ng dacite. nd subrounded clasts (up to 13 mm in hyric dacites, minor orange pink	Tpf			
960–985	Volcaniclastic sediments— gravels and fine- to mediu predominantly dacitic detri 960–970 ft WR: moderatel subangular to subrounded composed predominantly of dacite, trace vesicular rhyo fragments. 970–975 ft WR: fine to coa matrix. +10F: broken and s diameter) hbn- and tt-bear 975–980 ft WR: fine grave sandstone. +10F: subangu in diameter) composed of bt-bearing dacites.	-pale pinkish gray (7.5YR 7/1) fine m-grained sandstone with silt, tus. y silty matrix. +10F: broken and clasts (up to 6 mm in diameter) of hbn-dacites and minor bt-bearing odacite(?), and indurated sandstone urse sand with pebble gravel, silty subangular clasts (up to 8 mm in ing dacites. I and fine- to medium-grained ular to subrounded clasts (up to 10 mm light gray, pinkish and white hbn- and	Tpf			
985–990	Volcaniclastic sediments— gravels and medium to ver detritus. 985–990 ft +10F: subangu in diameter) composed of bt-bearing dacite.	-pale pinkish gray (7.5YR 7/1) coarse y coarse-grained sandstone, dacitic lar to subrounded clasts (up to 18 mm ight gray hbn-dacites and minor white	Tpf			
990–1005	Volcaniclastic sediments– very coarse grained sands detritus. 990–1005 ft +10F: broken diameter) composed predo gray hbn-dacites and lesse	-pale pinkish gray (7.5YR 7/1) fine- to tone with some pebble gravel, dacitic and subangular clasts (up to 15 mm in ominantly of coarsely porphyritic light or bt-bearing dacite.	Tpf	Estimated contact between Puye volcaniclastic sediments and underlying Miocene pumiceous sediments is placed at 1005 ft bgs.		

Borehole	ID: R-44	TA: 5	Page: 1	5 of 16
Depth (ft bgs)		Lithology	Lithologic Symbol	Notes
1005– 1010	MIOCENE PUMICEOUS S Pumiceous volcaniclastic s (GLEY1 7/0) to white (5YR granules, detritus of mixed 1005–110 ft WR/ +10F: bro clasts (up to 7 mm in diam fragments (glassy, phenoc minor aphyric rhyolite.	SEDIMENTS: sediments—varicolored light gray 8 8/1) fine to very coarse sand with pumice, and dacite. oken and subangular granule-size eter)composed of 60% pumice ryst-poor), 40% light gray dacite with	Tjfp	Miocene pumice-rich volcaniclastic sediments, encountered from 1005 to 1088 ft bgs, are estimated to be 83 ft thick.
1010– 1020	Pumiceous volcaniclastic s fine to coarse sand with per and dacitic detritus. 1010–1015 ft WR: modera fragments that are vitric ar pumices; 15%–20% gray of 1015–1020 ft +10F: broker 70%–75% white glassy ph in diameter); 25%–30% lig	sediments—pinkish white (5YR 8/2) ebble gravel and silt, mixed pumice, tely silty matrix. +10F: 100% pumice of phenocryst-poor. +35F: 80%–85% dacite grains. n and subangular clasts, enocryst-poor pumices (up to 10 mm ht gray dacite and white rhyolites.	Tifp	
1020– 1045	Pumiceous volcaniclastic s (5YR 8/2) to very light gray clasts) gravel and very coa and dacite. 1020–1025 ft WR/+10F: 60 poor pumice fragments; 10 dacite and lesser rhyodaci 7%–10% fragments of fine sandstone. 1025–1030 ft WR/+10F: 60 fragments; 15%–20% pink 10%–15% dacite clasts. 1030–1035 ft WR/+10F: 88 (rare quartz, biotite) pumic pumiceous sandstone frag 1035–1045 ft WR/+10f: 60 15%–20% pale pink tan pu 1%–20% subangular clasts volcanic detritus (dacite, a	sediments—varicolored pinkish white / (GLEY1 7/0) fine (i.e., pebble-size arse sand, mixed detritus of pumice, 0%–70% white glassy, phenocryst- 0%–20% subangular to subrounded te clasts (up to 13 mm in diameter); - to medium-grained pumiceous 5%–70% white glassy pumice tan pumiceous sandstone fragments; 5-95% white glassy phenocryst-poor e fragments; 5%–10% light pinkish tan ments; 3%–5% dacitic detritus. %–70% white glassy pumices; umiceous sandstone fragments, s (up to 12 mm in diameter) mixed ndesite, basalt).	Tifp	

Borehole	ID: R-44	TA: 5	Page: 16 of 16		
Depth (ft bgs)		Lithology	Lithologlc Symbol	Notes	
1045– 1075	Pumiceous volcaniclastic s (5YR 8/2) to medium gray pebble gravel mixed detritu of volcanic lithologies. 1045–1055 ft WR/+10F: 50 phenocryst-poor and biotit 30%–40% subangular class andesite and dacite; 3%–5 sandstone. 1055–1065 ft WR/+10F: 40 40%–50% mixed volcanic 1065–1070 ft WR/+10F: 50 30%–40% pinkish tan fine- pumice grains; 10–15% mi 1070–1075 ft WR: silty ma 25%–35% mixed volcanic sandstone fragments.	sediments—varicolored pinkish white (GLEY1 5/0) fine to coarse sand with us composed of pumice and a variety 0%–60% white vitric pumices (both e-bearing varieties present); sts (up to 10 mm in diameter) mixed % pink tan fragments pumiceous 0%–50% white vitric pumices; detritus (dacite, andesite, basalt). 0-60% white pumices; •grained sandstone with abundant tixed volcanic detritus. trix. +10F: 50%–60% white pumices; detritus; 10%–15% pumiceous	Tifp		
1075– 1088	Pumiceous volcaniclastic s 8/2), pale pinkish tan (5YR fine to medium gravels wit predominantly of dacite an 1075–1088 ft WR/+10F: 50 (up to 15 mm in diameter) 25%–30% white vitric pum pumiceous sandstone frag	sediments—varicolored, white (%YR 7/3) and medium gray (GLEY1 5/0) h fine to coarse sand, detritus d lesser pumices. 0%–60% broken and subangular clasts of dacite and minor andesite; ice fragments; 10%–20% indurated ments.	Tifp	Estimated contact between Miocene pumiceous sediments and underlying Miocene riverine sediments is placed at 1088 ft bgs.	
1088– 1094	MIOCENE RIVERINE SEL Axial-river gravel deposits- 5/0) to pink tan (5YR 7/4) f coarse sand, commonly ro diverse volcanic and Preca +10F: 20%–30% well-roun (up to 22 mm in diameter); volcanic clasts (up to 13 m fine-grained andesite and	DIMENTS: —varicolored medium gray (GLEY1 ine to coarse gravels with fine to unded detrital clasts composed of ambrian quartzo-feldspathic lithologies. ded quartzite and granitic pebbles 70%–80% broken and well rounded m in diameter) composed of dark gray varieties of dacite.	Tcar	Miocene riverine gravel deposits were encountered at the bottom of the R-44 borehole through the 6-ft interval, from 1088 to 1094 ft bgs (TD). Note: R-44 borehole drilling was concluded at a TD of 1094 ft bgs.	

ABBREVIATIONS

5YR 8/4 = Munsell rock color notation where hue (e.g., 5YR), value (e.g., 8), and chroma (e.g. 4) are expressed. Hue indicates soil color's relation to red, yellow, green, blue, and purple. Value indicates soil color's lightness. Chroma indicates soil color's strength.

% = estimated per cent by volume of a given sample constituent

bgs = below ground surface

bt = biotite

cpx = clinopyroxene

ft = feet

GM = groundmass

hbn = hornblende

N/S = no assigned symbol for geologic unit

ol = olivine

Qal = Quaternary Alluvium

Qbt 1g = vitric unit 1g of the Tshirege member of Bandelier Tuff

Qct = Cerro Toledo Interval

Qbo = Otowi Member of Bandelier Tuff

Qbog = Guaje Pumice Bed

Tb4 = Cerros del Rio Basalt

Tpf = Puye Formation

Y = Yellow

YR = Yellow red

- +10F = plus No. 10 sieve sample fraction
- +35F = plus No. 35 sieve sample fraction

Appendix B

Groundwater Analytical Results

B-1.0 SAMPLING AND ANALYSIS OF GROUNDWATER AT R-44

A total of 15 groundwater samples were collected at the regional aquifer well R-44; 11 samples during drilling and 4 samples during well development. Two groundwater samples potentially were collected from the vadose zone and 9 from the regional aquifer during drilling. The two vadose zone samples most likely consist of municipal water used during drilling, based on very small volumes of water produced from the borehole. In addition, low concentrations of key contaminants, including chloride, chromium, nitrate, and sulfate measured in the borehole samples, were not consistent with those measured at wells MCOI-4. MCOI-5, MCOI-6, SCI-1, SCI-2, R-28, and R-42. Perched intermediate-depth groundwater was not encountered during drilling at R-42 and R-28. The two vadose zone water samples were not analyzed for tritium, another key contaminant found in groundwater in Mortandad Canyon. The lack of tritium analysis on the two water samples places some small uncertainty on the occurrence of perched intermediate groundwater within the deep vadose zone at well R-44. During aguifer performance (pumping) testing, six groundwater samples were collected from screen 1 between a depth interval ranging from 895 to 905 ft below ground surface (bgs), and six groundwater samples were collected from screen 2 between a depth interval of 985 and 995 ft bgs. All of the groundwater samples were collected within the Puye Formation. The filtered samples were analyzed for cations, anions, perchlorate, and metals. A total of 16,005 gal. of groundwater was pumped from well R-44 during development before the aquifer tests. During the pumping tests conducted at well R-44, a total of 76,924 gal. of groundwater was pumped from screens 1 and 2.

B-1.1 Field Preparation and Analytical Techniques

Chemical analyses of groundwater-screening samples collected from well R-44 were performed at Los Alamos National Laboratory's (LANL's, or the Laboratory's) Earth and Environmental Sciences Group 14 (EES-14). Groundwater samples were filtered (0.45-µm membranes) before preservation and chemical analyses. Samples were acidified at the EES-14 wet chemistry laboratory with analytical grade nitric acid to a pH of 2.0 or less for metal and major cation analyses.

Groundwater samples were analyzed using techniques specified in the U.S. Environmental Protection Agency SW-846 manual. Ion chromatography (IC) was the analytical method for bromide, chloride, fluoride, nitrate, nitrite, oxalate, perchlorate, phosphate, and sulfate. The instrument detection limits for perchlorate were 0.002 and 0.005 ppm, depending on the sample type (borehole water versus developed well water) and analyte interferences due to the presence of drilling fluid (AQF-2) used during drilling. Inductively coupled (argon) plasma optical emission spectroscopy (ICPOES) was used for analyses of dissolved aluminum, barium, boron, calcium, total chromium, iron, lithium, magnesium, manganese, potassium, silica, sodium, strontium, titanium, and zinc. Dissolved aluminum, antimony, arsenic, barium, beryllium, boron, cadmium, cesium, chromium, cobalt, copper, iron, lead, lithium, manganese, mercury, molybdenum, nickel, rubidium, selenium, silver, thallium, thorium, tin, vanadium, uranium, and zinc were analyzed by inductively coupled (argon) plasma mass spectrometry (ICPMS). The precision limits (analytical error) for major ions and trace elements were generally less than ±7% using ICPOES and ICPMS. Concentrations of total organic carbon (TOC) in nonfiltered groundwater samples collected during well development and aquifer performance testing were determined by using an organic carbon analyzer. Charge balance errors for total cations and anions were generally less than $\pm 10\%$ for complete analyses of the above inorganic chemicals. The negative cation-anion charge balance values indicate excess anions for the filtered samples. Total carbonate alkalinity was measured using standard titration techniques.

B-1.2 Field Parameters

B-1.2.1 Well Development

Water samples were drawn from the pump flow line into sealed containers, and field parameters were measured using a YSI multimeter. Results of field parameters, consisting of pH, temperature, dissolved oxygen (DO), oxidation-reduction potential (ORP), specific conductance, and turbidity measured during well development at R-44, are provided in Table B-1.2.1-1. Seven measurements of pH and temperature varied from 8.22 to 8.30 and from 18.30°C to 18.56°C, respectively, in groundwater pumped from well R-44 screen 1 during development. Concentrations of DO ranged from 9.70 to 10.66 mg/L, and these anomalously high DO measurements suggest that the groundwater was aerated during field parameter measurements. Uncorrected ORP values varied from -135.1 to -129.7 millivolts (mV) during well development of R-44 screen 1 (Table B-1.2.1-1). These ORP measurements taken during well development are not considered to be reliable and representative of the known relatively oxidizing conditions characteristic of the regional aquifer beneath the Pajarito Plateau, based on analytical results for redox-sensitive solutes, including detectable chromium, nitrate, sulfate, and uranium provided in Table B-1.3.1-1. Measurable concentrations of these solutes are consistent with overall oxidizing conditions encountered at the well. Specific conductance ranged from 142 to 148 microsiemens per centimeter (µS/cm), and turbidity ranged from 0 to 0.1 nephelometric turbidity unit (NTU) during well development of R-44 screen 1 (Table B-1.2.1-1).

Thirteen measurements of pH and temperature varied slightly from 8.19 to 8.30 and from 17.47°C to 18.78°C, respectively, in groundwater pumped from well R-44 screen 2 during development (Table B-1.2.1-1). Concentrations of DO varied from 11.57 to 13.72 mg/L, and these anomalously high DO measurements suggest that the groundwater was aerated during field parameter measurements. Uncorrected ORP values varied from –130.8 to –118.9 mV (Table B-1.2.1-1) during well development of R-44 screen 2, which also are not consistent with analytical results for several of the redox-sensitive solutes listed above. The regional aquifer is relatively oxidizing beneath the Pajarito Plateau and positive, uncorrected ORP measurements are typically recorded at adjacent regional aquifer wells, including R-1, R-13, R-15, and R-28. Specific conductance ranged from 193 to 204 μS/cm in groundwater pumped from R-44 screen 2 during well development, and turbidity decreased from 55.8 to 0 NTUs. Eight of the 13 measurements had turbidity greater than 5 NTUs during well development of R-44 screen 2.

B-1.2.2 Aquifer Performance Testing

During aquifer performance testing, 29 measurements of pH and temperature varied from 7.80 to 8.04 and from 14.99°C to 19.08°C, respectively, at well R-44 screen 1 (Table B-1.2.1-1). Concentrations of DO varied from 7.95 to 9.30 mg/L and positive, uncorrected ORP values varied from 117.3 to 204.4 mV during aquifer performance testing of R-44 screen 1. The uncorrected ORP values are generally consistent with both the DO measurements and analytical results for redox-sensitive solutes listed above and are provided in Table B-1.3-1. Specific conductance ranged from 60 to 140 μ S/cm, and turbidity varied from 0 to 2.8 NTUs for groundwater pumped from R-44 screen 1 during aquifer performance testing.

Twenty-three measurements of pH and temperature varied from 8.31 to 8.67 and from 15.14°C to 20.31°C, respectively, during aquifer performance testing conducted at well R-44 screen 2. Concentrations of DO ranged from 8.60 to 11.14 mg/L. The anomalously high DO concentrations (greater than 9 mg/L) suggest that the water samples were aerated during parameter measurement. Positive, uncorrected ORP values varied from 144.9 to 212.1 mV during aquifer performance testing of R-44 screen 2. Specific conductance decreased from 173 to 154 μ S/cm for the R-44 screen 2 samples measured during aquifer performance testing. Turbidity varied from 1.6 to 5.9 NTUs with one turbidity value greater than 5 NTUs.

B-1.3 Analytical Results for R-44 Groundwater-Screening Samples

B-1.3.1 Well Development

Analytical results for groundwater-screening samples collected at well R-44 during drilling, well development, and aquifer performance testing are provided in Table B-1.3.1-1. Four groundwater samples were collected from R-44 screens 1 and 2 during well development, and selected analytical results for these samples are combined in the following discussion. Calcium and sodium are the dominant cations in regional aquifer groundwater pumped from well R-44. During well development, dissolved concentrations of calcium and sodium ranged from 12.29 to 13.01 ppm (12.29 to 13.01 mg/L) and from 11.61 to 30.05 ppm, respectively. Dissolved concentrations of chloride and fluoride varied from 4.84 to 8.13 ppm and from 0.39 to 0.42 ppm, respectively, during development conducted at well R-44 (Table B-1.3.1-1). Dissolved concentrations of nitrate(N) and sulfate ranged from 0.57 to 1.01 ppm and from 5.83 to 13.8 ppm, respectively, during development at well R-44. Dissolved concentrations of chloride, nitrate(N), and sulfate exceeded Laboratory median background for regional aquifer groundwater (LANL 2007, 095817). Median background concentrations for dissolved chloride, nitrate plus nitrite(N), and sulfate in the regional aquifer are 2.17 mg/L, 0.31 mg/L, and 2.83 mg/L, respectively (LANL 2007, 095817). Concentrations of TOC ranged from 0.55 to 0.71 mgC/L in groundwater-screening samples collected during development conducted at well R-44 (Table B-1.3.1-1). The median background concentration of TOC is 0.34 mgC/L for regional aquifer groundwater (LANL 2007, 095817). Concentrations of perchlorate were less than analytical detection (<0.002 ppm, IC method) in groundwater-screening samples collected from well R-44 during development (Table B-1.3.1-1).

During well development conducted at R-44, dissolved concentrations of iron ranged from 0.180 to 0.430 ppm (180 to 430 µg/L or 180 to 430 ppb) using ICPOES (Table B-1.3.1-1), which exceeded the maximum background value of 147 µg/L for regional aquifer groundwater (LANL 2007, 095817). Dissolved concentrations of manganese ranged from 0.011 to 0.019 ppm (Table B-1.3.1-1), which exceeded the median background value of 1.0 µg/L for regional aquifer groundwater (LANL 2007, 095817). A carbon-steel discharge pipe was used during well development at R-44, which contributed iron and manganese in the form of colloidal rust to the filtered groundwater samples. Dissolved concentrations of boron ranged from 0.006 to 0.023 ppm (Table B-1.3.1-1) at well R-44, which is below the maximum background value of 51.6 µg/L for the regional aquifer (LANL 2007, 095817). Dissolved concentrations of nickel were less than analytical detection (0.001 ppm, ICPMS method) (Table B-1.3.1-1) in four groundwater-screening samples collected during well development conducted at R-44. Dissolved concentrations of zinc ranged from 0.003 to 0.007 ppm in groundwater-screening samples collected at well R-44 during development (Table B-1.3.1-1). The background median concentration of zinc in filtered samples is 1.45 µg/L for the regional aguifer (LANL 2007, 095817). Total dissolved concentrations of chromium ranged from 0.004 to 0.008 ppm (4 to 8 µg/L) at well R-44 during well development, with the higher concentrations of this metal measured in groundwater samples collected from screen 1 (Table B-1.3.1-1). Background mean, median, and maximum concentrations of total dissolved chromium are 3.07 μ g/L, 3.05 μ g/L, and 7.20 μ g/L, respectively, for the regional aquifer (LANL 2007, 095817).

B-1.3-2 Aquifer Performance Testing

Dissolved concentrations of calcium and sodium ranged from 11.54 to 12.0 ppm and from 8.65 to 9.74 ppm, respectively, during aquifer performance testing conducted at R-44 screen 1 (Table B-1.3.1-1). Dissolved concentrations of chloride and fluoride varied from 3.29 to 3.44 ppm and from 0.36 to 0.37 ppm, respectively, during this phase of testing conducted at well R-44 screen 1 (Table B-1.3.1-1). Dissolved concentrations of nitrate(N) and sulfate varied slightly from 1.12 to 1.14 ppm and from 4.20 to 4.44 ppm, respectively, during aquifer performance testing performed at well R-44 screen 1. Dissolved

concentrations of chloride, nitrate(N), and sulfate in groundwater-screening samples collected from R-44 screen 1 exceeded Laboratory median background within regional aquifer groundwater (LANL 2007, 095817). Median background concentrations for dissolved chloride, nitrate plus nitrite(N), and sulfate in the regional aquifer are 2.17 mg/L, 0.31 mg/L, and 2.83 mg/L, respectively (LANL 2007, 095817). Elevated above-background concentrations of chloride, nitrate(N), and sulfate at well R-44 screen 1 suggest the presence of a contaminant plume(s) consisting, in part, of treated sewage effluent most likely released from Technical Area 03 (TA-03) discharges and possibly from other sewage/industrial waste streams released within Mortandad Canyon. Concentrations of TOC measured in groundwater-screening samples were 0.50 mgC/L during aquifer performance testing conducted at well R-44 screen 1 (Table B-1.3.1-1). Concentrations of perchlorate were less than detection (<0.002 ppm, IC method) in groundwater-screening samples collected from well R-44 screen 1 during aquifer performance testing (Table B-1.3.1-1).

During aguifer performance testing at R-44 screen 1, dissolved concentrations of iron were generally less than analytical detection (0.010 ppm) using ICPOES (Table B.1-3-1). A stainless-steel discharge pipe was used during aquifer performance testing conducted at R-44 screens 1 and 2, which is much less corrodible than the carbon steel used during development. Dissolved concentrations of manganese varied slightly from 0.002 to 0.003 ppm (Table B-1.3.1-1 at well R-44 screen 1 during this phase of testing. Dissolved concentrations of boron ranged from 0.013 to 0.018 ppm (Table B-1.3.1-1) in groundwater-screening samples collected from well R-44 screen 1, which is below the maximum background value of 51.6 μ g/L for the regional aguifer (LANL 2007, 095817). Dissolved concentrations of nickel were less than analytical detection (0.001 ppm, ICPMS method) (Table B-1.3.1-1) in six groundwater-screening samples collected from R-44 screen 1 during aquifer performance testing. Dissolved concentrations of zinc ranged from 0.005 to 0.0013 ppm in groundwater-screening samples collected from R-44 screen 1 during this phase of testing (Table B-1.3.1-1). The background median concentration of zinc in filtered samples is 1.45 µg/L for the regional aquifer (LANL 2007, 095817). Total dissolved concentrations of chromium were 0.014 ppm (14 µg/L) in six aroundwater-screening samples collected from R-44 screen 1 during aguifer performance testing (Table B-1.3.1-1). Background mean, median, and maximum concentrations of total dissolved chromium are 3.07 µg/L, 3.05 µg/L, and 7.20 µg/L, respectively, for the regional aquifer (LANL 2007, 095817). The most likely source of dissolved chromium measured in groundwater samples collected from well R-44 screen 1 is from past releases associated with the TA-03 cooling towers, in which potassium dichromate was used as a corrosion inhibitor from 1956 to 1972. Chromate (CrO_4^{2-}) is mobile in groundwater under oxidizing and basic pH conditions characteristic of most perched intermediate saturated zones and the regional aquifer at Los Alamos.

During aquifer performance testing of R-44 screen 2, dissolved concentrations of calcium and sodium ranged from 12.82 to 13.49 ppm and from 11.46 to 15.27 ppm, respectively, which are slightly higher than those measured in groundwater-screening samples collected from R-44 screen 1. Dissolved concentrations of chloride and fluoride varied slightly from 3.62 to 4.31 ppm and from 0.40 to 0.42 ppm, respectively, during aquifer performance testing conducted at well R-44 screen 2 (Table B-1.3.1-1). Dissolved concentrations of nitrate(N) varied slightly from 0.60 to 0.62 ppm, which are less than dissolved concentrations of nitrate(N) measured in groundwater-screening samples collected from R-44 screen 1. Dissolved concentrations of sulfate decreased from 7.38 to 4.71 ppm during aquifer performance testing conducted at well R-44 screen generations of chloride, nitrate(N), and sulfate at well R-44 exceeded Laboratory median background within regional aquifer groundwater (LANL 2007, 095817). Concentrations of TOC were 0.50 mgC/L during aquifer performance testing conducted at well R-44 screen 2 (Table B-1.3.1-1). Concentrations of perchlorate were less than detection (<0.002 ppm, IC method) in groundwater-screening samples collected from well R-44 screen 2 during aquifer performance testing conducted at well R-44 screen 1. Dissolved concentrations of perchlorate were less than detection (Table B-1.3.1-1).

During aquifer performance testing conducted at R-44 screen 2, dissolved concentrations of iron were generally less than analytical detection (0.010 ppm) using ICPOES (Table B-1.3.1-1). Dissolved concentrations of manganese varied slightly from 0.007 to 0.008 ppm (Table B-1.3.1-1) at well R-44 screen 2. Dissolved concentrations of boron ranged from 0.014 to 0.020 ppm (Table B-1.3.1-1) at well R-44 screen 2, which is below the maximum background value of 51.6 μ g/L for the regional aquifer (LANL 2007, 095817). Dissolved concentrations of boron are similar in groundwater-screening samples collected from both screens at R-44 (Table B-1.3.1-1). Detectable dissolved concentrations of nickel were 0.002 ppm in groundwater-screening samples collected from R-44 screen 2 during aquifer performance testing (Table B-1.3.1-1). Dissolved concentrations of zinc varied slightly from 0.005 to 0.006 ppm in groundwater-screening samples collected from R-44 screen 2 during aquifer performance testing (Table B-1.3.1-1). Total dissolved concentrations of chromium ranged from 0.004 to 0.006 ppm (4 to 6 μ g/L) at well R-44 screen 2 (Table B-1.3.1-1). Background mean, median, and maximum concentrations of total dissolved chromium are 3.07 μ g/L, 3.05 μ g/L, and 7.20 μ g/L, respectively, for the regional aquifer (LANL 2007, 095817). Total dissolved concentrations of chromium are lower in groundwater-screening samples collected from screen 1 at well R-44.

B-2.0 REFERENCES

The following list includes all documents cited in this appendix. Parenthetical information following each reference provides the author(s), publication date, and ER ID number. This information is also included in text citations. ER ID numbers are assigned by the Environmental Programs Directorate's Records Processing Facility (RPF) and are used to locate the document at the RPF and, where applicable, in the master reference set.

Copies of the master reference set are maintained at the NMED Hazardous Waste Bureau and the Directorate. The set was developed to ensure that the administrative authority has all material needed to review this document, and it is updated with every document submitted to the administrative authority. Documents previously submitted to the administrative authority are not included.

LANL (Los Alamos National Laboratory), May 2007. "Groundwater Background Investigation Report, Revision 3," Los Alamos National Laboratory document LA-UR-07-2853, Los Alamos, New Mexico. (LANL 2007, 095817)

Date	рН	Temp (°C)	DO (mg/L)	ORP (mV)	Specific Conductivity (µS/cm)	Turbidity (NTU)	Purge Volume between Samples (gal.)	Cumulative Purge Volume (gal.)
Well Develo	oment							
01/15/09	n/r*, bailir	ng					500	500
01/16/09	n/r, bailing	9					300	800
01/17/09	n/r, pump	ing					3000	3800
01/18/09	n/r, pump	ing					1785	5585
	8.30	18.30	10.66	-133.6	148	0.1	1150	6735
	8.27	18.33	9.88	-133.7	145	0.0	235	6970
01/18/09	8.26	18.35	10.12	-133.0	144	0.0	235	7205
(upper	8.26	18.42	11.02	-132.9	144	0.0	235	7440
screen)	8.25	18.56	9.77	-135.1	144	0.0	235	7675
	8.23	18.47	9.70	-133.3	144	0.0	235	7910
	8.22	18.48	10.30	-129.7	142	0.0	235	8145
01/19/09	n/r, pump	ing					2340	10,485
01/20/09	n/r, pump	ing					2610	13,095
	8.29	17.47	13.65	-125.2	204	55.8	664	13,759
	8.29	17.49	13.72	-118.9	204	43.2	246	14,005
	8.26	17.55	12.96	-121.4	201	35.9	248	14,253
	8.26	17.63	12.84	-120.8	199	15.4	248	14,501
	8.25	17.67	12.91	-120.2	198	22.2	248	14,749
01/20/09	8.22	17.75	12.50	-122.5	195	14.8	248	14,997
(lower	8.20	18.03	11.94	-129.9	195	7.2	144	15,141
screen)	8.20	18.45	12.07	-130.6	196	7.1	144	15,285
	8.22	18.58	12.00	-130.5	195	4.7	144	15,429
	8.23	18.65	11.64	-130.8	195	1.2	144	15,573
	8.22	18.68	12.03	-130.4	194	0.0	144	15,717
	8.20	18.75	11.57	-130.2	193	0.0	144	15,861
	8.19	18.78	12.48	-129.9	193	0.0	144	16,005

 Table B-1.2.1-1

 Well Development Volumes, Aquifer Pump Test Volumes, and Associated Field Water-Quality Parameters for R-44

						,		
Date	рН	Temp (°C)	DO (mg/L)	ORP (mV)	Specific Conductivity (µS/cm)	Turbidity (NTU)	Purge Volume between Samples (gal.)	Cumulative Purge Volume (gal.)
Aquifer Pum	ping Test	Volumes						
02/14/09	n/r, pump	ing, step-te	est upper s	screen			3312	3312
02/15/09	n/r, pump	ing, step-te	est upper s	screen			145	3457
	7.80	17.94	8.59	121.0	140	0.7	724	4181
	8.04	16.50	8.78	117.3	130	0.2	724	4905
	7.97	16.67	9.00	158.9	130	0.0	724	5629
	7.96	17.27	8.81	156.7	130	0.1	725	6354
	7.89	18.55	8.96	162.6	70	0.3	1448	7802
	7.98	18.72	8.67	170.8	130	0.5	1449	9251
	7.97	17.18	8.71	168.7	130	0.4	1448	10,699
	8.00	17.65	8.90	171.5	130	0.4	1449	12,148
	8.01	16.42	9.18	189.7	130	0.5	1448	13,596
	7.94	15.65	8.81	178.9	130	0.1	1449	15,045
	7.96	14.99	9.07	183.6	130	0.2	1448	16,493
	8.00	17.25	8.59	189.0	130	0.1	1449	17,942
	7.98	17.18	9.30	184.1	130	1.1	1448	19,390
02/16-17/09	7.97	17.01	8.35	204.4	130	0.1	5794	25,184
(upper	7.98	17.12	9.29	199.6	130	0.2	1448	26,632
screen)	7.99	16.85	8.25	194.4	60	0.2	724	27,356
	8.03	17.69	8.17	195.2	120	2.8	724	28,080
	7.99	16.15	8.56	192.9	60	0.3	1448	29,528
	7.98	17.21	8.32	193.7	110	0.2	1449	30,977
	8.01	17.16	8.26	192.6	130	0.1	1448	32,425
	7.98	18.37	8.16	151.3	120	0.4	1449	33,874
	8.02	18.86	8.03	161.9	130	0.5	1448	35,322
	8.01	18.41	8.01	169.7	120	0.4	1449	36,771
	8.02	18.79	8.18	179.4	67	0.1	242	37013
	8.02	18.68	8.14	184.9	130	0.2	242	37,255
	8.01	18.65	7.98	188.4	130	0.2	242	37,497
	8.03	18.70	7.95	188.9	130	0.1	242	37,739
	8.03	18.55	8.04	189.9	130	0.1	242	37,981
	8.01	19.08	8.16	189.4	130	0.2	242	38,223
02/19/09	n/r, pump	ing, step-te	ests lower	screen			4275	42,498
	8.31	17.47	8.72	145.7	173	5.0	956	43,454
02/21-22/09	8.35	15.14	10.64	157.6	167	4.3	478	43,932
(lower	8.67	18.72	8.90	144.9	173	3.1	1434	45,366
screen)	8.66	19.50	10.38	148.7	171	4.0	1434	46,800
	8.65	19.41	8.79	149.0	168	5.9	1434	48,234

Table B-1.2.1-1 (Continued)

Date	рН	Temp (°C)	DO (mg/L)	ORP (mV)	Specific Conductivity (µS/cm)	Turbidity (NTU)	Purge Volume between Samples (gal.)	Cumulative Purge Volume (gal.)
	8.62	19.60	9.05	180.7	167	4.7	1434	49,668
	8.62	19.84	9.06	165.6	166	3.0	1434	51,102
	8.63	20.31	8.60	166.8	164	3.1	1434	52,536
	8.61	19.96	9.70	167.1	163	3.5	1434	53,970
	8.60	19.06	9.34	163.2	161	4.7	1434	55,404
	8.67	16.59	9.26	176.2	156	2.4	1434	56,838
	8.63	17.12	9.12	179.9	157	1.8	1434	58,272
	8.64	17.81	11.14	182.4	152	4.0	4302	62,574
	8.62	17.82	9.55	187.4	158	4.0	1434	64,008
	8.60	17.66	10.45	192.4	157	3.8	1434	65,442
	8.57	18.96	10.47	161.2	156	2.5	1434	66,876
	8.58	18.56	10.77	189.3	157	3.5	1434	68,310
	8.56	n/r	9.18	191.2	156	2.2	1434	69,744
	8.60	n/r	10.11	192.9	155	2.1	1434	71,178
	8.62	n/r	10.32	192.9	157	3.0	1434	72,612
	8.63	n/r	8.73	194.8	154	1.7	1434	74,046
	8.61	19.39	8.47	212.1	155	1.6	1434	75,480
	8.62	19.27	8.62	197.8	155	1.6	1195	76,675
	n/r						249	76,924

Table B-1.2.1-1 (Continued)

Note: Cumulative purge volumes for pump test calculated using average pump discharge rate of 24.1gal./min in the upper screen and 23.9 1 gal./min in the lower screen.

* n/r = Not recorded.

 Table B-1.3.1-1

 Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	ER/RRES-WQH	Screen	Depth (ft)	Ag rslt (ppm)	stdev (Ag)	Al rslt (ppm)	stdev (AI)	As rslt (ppm)	stdev (As)	B rslt (ppm)	stdev (B)	Ba rslt (ppm)
GW44-09-1292	11/17/2008	Not applicable	Borehole	09-329	Not applicable	739	0.001	U	0.21	0.00	0.0011	0.0000	0.038	0.001	0.015
GW44-09-1293	12/8/2008	Not applicable	Borehole	09-453	Not applicable	920	0.001	U	0.43	0.00	0.0005	0.0000	0.079	0.001	0.456
GW44-09-1294	12/8/2008	Not applicable	Borehole	09-453	Not applicable	937	0.001	U	0.42	0.00	0.0012	0.0000	0.088	0.001	0.453
GW44-09-1295	12/8/2008	Not applicable	Borehole	09-453	Not applicable	977	0.001	U	0.30	0.00	0.0003	0.0000	0.093	0.001	0.383
GW44-09-1296	12/8/2008	Not applicable	Borehole	09-453	Not applicable	997	0.001	U	1.39	0.01	0.0018	0.0002	0.076	0.001	0.418
GW44-09-1297	12/8/2008	Not applicable	Borehole	09-453	Not applicable	1017	0.001	U	1.54	0.06	0.0011	0.0002	0.118	0.004	0.509
GW44-09-1298	12/8/2008	Not applicable	Borehole	09-453	Not applicable	1037	0.001	U	0.98	0.02	0.0007	0.0000	0.076	0.002	0.386
GW44-09-1299	12/8/2008	Not applicable	Borehole	09-453	Not applicable	1056	0.001	U	0.93	0.00	0.0009	0.0000	0.073	0.001	0.271
GW44-09-1300	12/9/2008	Not applicable	Borehole	09-473	Not applicable	1094	0.001	U	0.02	0.00	0.0011	0.0000	0.070	0.001	0.434
GW44-09-1301	12/9/2008	Not applicable	Borehole	09-473	Not applicable	1076	0.001	U	0.29	0.01	0.0004	0.0000	0.147	0.001	0.536
GW44-09-1272	1/20/2009	Not applicable	Well, development	09-658	1	895-905	0.001	U	0.004	0.000	0.0008	0.0000	0.009	0.000	0.031
GW44-09-1273	1/20/2009	Not applicable	Well, development	09-658	1	895-905	0.001	U	0.004	0.000	0.0007	0.0000	0.006	0.001	0.027
GW44-09-1274	1/20/2009	Not applicable	Well, development	09-679	2	985-995	0.001	U	0.022	0.000	0.0016	0.0000	0.023	0.001	0.056
GW44-09-1275	1/20/2009	Not applicable	Well, development	09-679	2	985-995	0.001	U	0.006	0.000	0.0013	0.0000	0.017	0.000	0.048
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	Not provided	1	895-905	0.001	U	0.005	0.000	0.0008	0.0000	0.014	0.000	0.025
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	Not provided	1	895-905	0.001	U	0.004	0.000	0.0007	0.0000	0.018	0.001	0.024
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	Not provided	1	895-905	0.001	U	0.007	0.000	0.0007	0.0000	0.016	0.001	0.025
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	Not provided	1	895-905	0.001	U	0.005	0.000	0.0008	0.0000	0.014	0.000	0.024
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	Not provided	1	895-905	0.001	U	0.006	0.000	0.0008	0.0000	0.014	0.000	0.023
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	Not provided	1	895-905	0.001	U	0.005	0.000	0.0007	0.0000	0.013	0.001	0.022
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	09-972	2	985-995	0.001	U	0.005	0.000	0.0010	0.0000	0.020	0.001	0.031
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	09-972	2	985-995	0.001	U	0.011	0.000	0.0009	0.0000	0.018	0.000	0.029
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	09-972	2	985-995	0.001	U	0.005	0.000	0.0009	0.0000	0.016	0.000	0.028
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	09-972	2	985-995	0.001	U	0.005	0.000	0.0009	0.0000	0.015	0.001	0.028
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	09-972	2	985-995	0.001	U	0.006	0.000	0.0009	0.0000	0.014	0.000	0.027
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	09-972	2	985-995	0.001	U	0.005	0.000	0.0009	0.0000	0.014	0.000	0.027

Notes: U = Not detected. Total organic carbon not analyzed in borehole samples collected in high-density polyethylene containers.

 Table B-1.3.1-1

 Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	stdev (Ba)	Be rslt (ppm)	stdev (Be)	Br(-) ppm	TOC rslt (ppm)	TOC (U)	Ca rslt (ppm)	stdev (Ca)	Cd rslt (ppm)	stdev (Cd)	CI(-) ppm	CIO4(-) ppm	CIO4(-) (U)	Corslt (ppm)
GW44-09-1292	11/17/2008	Not applicable	Borehole	0.000	0.001	U	0.15	Not analyzed		13.77	0.08	0.001	U	7.07	0.005	U	0.001
GW44-09-1293	12/8/2008	Not applicable	Borehole	0.002	0.001	U	0.05	Not analyzed		14.46	0.10	0.001	U	7.89	0.005	U	0.001
GW44-09-1294	12/8/2008	Not applicable	Borehole	0.006	0.001	U	0.04	Not analyzed		14.79	0.17	0.001	U	7.24	0.005	U	0.001
GW44-09-1295	12/8/2008	Not applicable	Borehole	0.003	0.001	U	0.05	Not analyzed		10.59	0.05	0.001	U	5.30	0.005	U	0.001
GW44-09-1296	12/8/2008	Not applicable	Borehole	0.002	0.001	U	0.05	Not analyzed		11.64	0.01	0.001	U	7.87	0.005	U	0.001
GW44-09-1297	12/8/2008	Not applicable	Borehole	0.001	0.001	U	0.03	Not analyzed		11.83	0.04	0.001	U	4.62	0.002	υ	0.001
GW44-09-1298	12/8/2008	Not applicable	Borehole	0.002	0.001	U	0.04	Not analyzed		11.62	0.09	0.001	U	3.65	0.002	U	0.001
GW44-09-1299	12/8/2008	Not applicable	Borehole	0.001	0.001	U	0.04	Not analyzed		11.57	0.05	0.001	U	3.76	0.002	U	0.001
GW44-09-1300	12/9/2008	Not applicable	Borehole	0.002	0.001	U	0.04	Not analyzed		21.75	0.07	0.001	U	3.94	0.005	U	0.001
GW44-09-1301	12/9/2008	Not applicable	Borehole	0.002	0.001	U	0.04	Not analyzed		9.13	0.04	0.001	U	4.62	0.005	U	0.001
GW44-09-1272	1/20/2009	Not applicable	Well, development	0.000	0.001	U	0.07	0.55		13.01	0.04	0.001	U	5.05	0.002	U	0.001
GW44-09-1273	1/20/2009	Not applicable	Well, development	0.000	0.001	U	0.07	0.66		12.95	0.02	0.001	U	4.84	0.002	U	0.001
GW44-09-1274	1/20/2009	Not applicable	Well, development	0.000	0.001	U	0.07	0.70		12.29	0.04	0.001	U	8.13	0.002	U	0.001
GW44-09-1275	1/20/2009	Not applicable	Well, development	0.001	0.001	U	0.06	0.71		12.84	0.14	0.001	U	7.39	0.002	U	0.001
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	11.78	0.04	0.001	U	3.44	0.002	U	0.001
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	0.000	0.001	U	0.04	0.50	U	11.54	0.03	0.001	U	3.35	0.002	U	0.001
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	11.82	0.06	0.001	U	3.34	0.002	U	0.001
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	12.00	0.13	0.001	U	3.33	0.002	U	0.001
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	0.001	0.001	U	0.03	0.50	U	11.71	0.10	0.001	U	3.29	0.002	U	0.001
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	0.000	0.001	U	0.02	0.50	U	11.69	0.05	0.001	U	3.30	0.002	U	0.001
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	12.82	0.11	0.001	U	4.31	0.002	U	0.001
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	13.11	0.08	0.001	U	4.09	0.002	U	0.001
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	13.24	0.02	0.001	U	3.91	0.002	U	0.001
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	13.33	0.10	0.001	U	3.83	0.002	U	0.001
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	13.40	0.05	0.001	U	3.62	0.002	U	0.001
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	0.000	0.001	U	0.03	0.50	U	13.49	0.09	0.001	U	3.65	0.002	U	0.001

 Table B-1.3.1-1

 Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	stdev (Co)	Alk-CO3 rslt (ppm)	ALK-CO3 (U)	Cr rslt (ppm)	stdev (Cr)	Cs rslt (ppm)	stdev (Cs)	Cu rslt (ppm)	stdev (Cu)	F(-) ppm	Fe rslt (ppm)	stdev (Fe)
GW44-09-1292	11/17/2008	Not applicable	Borehole	U	0.8	U	0.005	0.000	0.001	U	0.003	0.000	0.3	0.54	0.01
GW44-09-1293	12/8/2008	Not applicable	Borehole	U	0.8	U	0.001	0.000	0.001	U	0.002	0.000	0.79	0.51	0.00
GW44-09-1294	12/8/2008	Not applicable	Borehole	U	0.8	U	0.001	0.000	0.001	U	0.001	0.000	1.08	0.23	0.00
GW44-09-1295	12/8/2008	Not applicable	Borehole	U	0.8	U	0.001	0.000	0.001	U	0.001	U	0.66	0.18	0.00
GW44-09-1296	12/8/2008	Not applicable	Borehole	U	0.8	U	0.003	0.001	0.001	U	0.003	0.001	0.81	0.83	0.03
GW44-09-1297	12/8/2008	Not applicable	Borehole	U	0.8	U	0.002	0.000	0.001	U	0.002	0.000	0.81	0.68	0.23
GW44-09-1298	12/8/2008	Not applicable	Borehole	U	0.8	U	0.005	0.001	0.001	U	0.002	0.000	0.44	2.59	0.03
GW44-09-1299	12/8/2008	Not applicable	Borehole	U	0.8	U	0.002	0.000	0.001	U	0.002	0.000	0.63	1.13	0.02
GW44-09-1300	12/9/2008	Not applicable	Borehole	U	6.89		0.009	0.001	0.001	U	0.001	U	0.67	0.02	0.00
GW44-09-1301	12/9/2008	Not applicable	Borehole	U	0.8	U	0.006	0.000	0.001	U	0.001	U	0.69	0.20	0.00
GW44-09-1272	1/20/2009	Not applicable	Well, development	U	0.8	U	0.008	0.000	0.001	U	0.001	U	0.39	0.24	0.00
GW44-09-1273	1/20/2009	Not applicable	Well, development	U	0.8	U	0.008	0.000	0.001	U	0.001	U	0.39	0.23	0.00
GW44-09-1274	1/20/2009	Not applicable	Well, development	U	0.8	U	0.004	0.001	0.001	U	0.001	U	0.42	0.18	0.00
GW44-09-1275	1/20/2009	Not applicable	Well, development	U	0.8	U	0.004	0.001	0.001	U	0.001	U	0.41	0.43	0.01
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	U	0.8	U	0.014	0.000	0.001	U	0.001	U	0.36	0.01	U
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	U	0.8	U	0.014	0.000	0.001	U	0.001	U	0.36	0.01	U
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	U	0.8	U	0.014	0.000	0.001	U	0.001	U	0.36	0.01	0.00
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	U	0.8	U	0.014	0.000	0.001	U	0.001	U	0.37	0.01	U
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	U	0.8	U	0.014	0.000	0.001	U	0.001	U	0.36	0.01	U
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	U	0.8	U	0.014	0.001	0.001	U	0.001	U	0.37	0.01	U
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	U	0.8	U	0.005	0.000	0.001	U	0.001	U	0.40	0.01	U
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	U	0.8	U	0.005	0.000	0.001	U	0.001	U	0.41	0.03	0.00
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	U	0.8	U	0.004	0.000	0.001	U	0.001	U	0.42	0.01	U
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	U	0.8	U	0.005	0.001	0.001	U	0.001	U	0.41	0.01	U
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	U	0.8	U	0.006	0.000	0.001	U	0.001	U	0.40	0.01	U
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	U	0.8	U	0.005	0.000	0.001	U	0.001	U	0.41	0.01	0.00

 Table B-1.3.1-1

 Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	Alk-CO3+HCO3 rslt (ppm)	Hg rslt (ppm)	stdev (Hg)	K rslt (ppm)	stdev (K)	Li rslt (ppm)	stdev (Li)	Mg rslt (ppm)	stdev (Mg)	Mn rslt (ppm)	stdev (Mn)
GW44-09-1292	11/17/2008	Not applicable	Borehole	128	0.00005	U	2.12	0.03	0.030	0.001	4.38	0.06	0.059	0.000
GW44-09-1293	12/8/2008	Not applicable	Borehole	85	0.00108	0.00004	2.71	0.01	0.037	0.000	4.58	0.01	0.198	0.001
GW44-09-1294	12/8/2008	Not applicable	Borehole	99	0.00210	0.00004	3.05	0.06	0.036	0.001	4.51	0.06	0.080	0.006
GW44-09-1295	12/8/2008	Not applicable	Borehole	68	0.00017	0.00000	2.06	0.02	0.035	0.000	3.31	0.03	0.075	0.002
GW44-09-1296	12/8/2008	Not applicable	Borehole	85	0.00062	0.00003	2.35	0.02	0.032	0.000	4.72	0.04	0.044	0.007
GW44-09-1297	12/8/2008	Not applicable	Borehole	88	0.00299	0.00012	2.31	0.01	0.048	0.000	4.28	0.03	0.033	0.004
GW44-09-1298	12/8/2008	Not applicable	Borehole	81	0.00012	0.00001	1.66	0.01	0.028	0.000	4.04	0.03	0.090	0.001
GW44-09-1299	12/8/2008	Not applicable	Borehole	91	0.00078	0.00002	3.51	0.02	0.051	0.000	4.53	0.02	0.067	0.002
GW44-09-1300	12/9/2008	Not applicable	Borehole	138	0.00148	0.00002	2.11	0.01	0.048	0.003	6.32	0.05	0.008	0.001
GW44-09-1301	12/9/2008	Not applicable	Borehole	83	0.00173	0.00003	1.59	0.00	0.039	0.002	3.27	0.01	0.028	0.001
GW44-09-1272	1/20/2009	Not applicable	Well, development	84	0.00005	U	1.20	0.02	0.023	0.000	3.62	0.08	0.013	0.000
GW44-09-1273	1/20/2009	Not applicable	Well, development	83	0.00005	U	1.08	0.02	0.021	0.000	3.36	0.04	0.011	0.000
GW44-09-1274	1/20/2009	Not applicable	Well, development	113	0.00005	U	1.68	0.01	0.030	0.000	4.23	0.02	0.018	0.000
GW44-09-1275	1/20/2009	Not applicable	Well, development	107	0.00005	U	1.56	0.03	0.028	0.001	4.25	0.08	0.019	0.000
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	82	0.00005	U	1.07	0.01	0.021	0.000	3.37	0.02	0.003	0.000
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	78	0.00005	U	1.07	0.01	0.021	0.000	3.31	0.03	0.002	0.000
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	77	0.00005	U	1.07	0.01	0.021	0.000	3.38	0.02	0.002	0.000
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	78	0.00005	U	1.09	0.01	0.022	0.000	3.45	0.02	0.002	0.000
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	81	0.00005	U	1.05	0.01	0.021	0.000	3.32	0.01	0.001	0.000
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	79	0.00005	U	1.02	0.00	0.020	0.000	3.23	0.02	0.001	0.000
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	91	0.00005	U	1.35	0.00	0.022	0.000	3.66	0.02	0.008	0.000
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	88	0.00005	U	1.35	0.01	0.022	0.000	3.80	0.03	0.008	0.000
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	87	0.00005	U	1.33	0.01	0.022	0.000	3.85	0.02	0.007	0.000
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	87	0.00005	U	1.33	0.01	0.022	0.000	3.89	0.02	0.007	0.000
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	86	0.00005	U	1.29	0.01	0.022	0.000	3.87	0.02	0.007	0.000
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	85	0.00005	U	1.30	0.01	0.022	0.000	3.91	0.00	0.007	0.000

 Table B-1.3.1-1

 Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	Mo rslt (ppm)	stdev (Mo)	Na rslt (ppm)	stdev (Na)	Ni rslt (ppm)	stdev (Ni)	NO2(ppm)	NO2-N rslt	NO2-N (U)	NO3 ppm	NO3-N rslt	C2O4 rslt (ppm)
GW44-09-1292	11/17/2008	Not applicable	Borehole	0.069	0.000	26.96	0.27	0.003	0.000	0.04	0.01	0.001	0.46	0.10	0.08
GW44-09-1293	12/8/2008	Not applicable	Borehole	0.238	0.002	16.53	0.07	0.002	0.000	0.01	0.00	U	6.36	1.44	0.58
GW44-09-1294	12/8/2008	Not applicable	Borehole	0.243	0.002	17.32	0.20	0.001	0.000	0.01	0.00	U	3.64	0.82	0.52
GW44-09-1295	12/8/2008	Not applicable	Borehole	0.145	0.001	13.78	0.06	0.001	0.000	0.01	0.00	U	7.22	1.63	0.44
GW44-09-1296	12/8/2008	Not applicable	Borehole	0.052	0.002	13.33	0.06	0.002	0.000	0.01	0.00	U	6.38	1.44	0.61
GW44-09-1297	12/8/2008	Not applicable	Borehole	0.049	0.001	14.85	0.07	0.001	0.000	0.01	0.00	U	2.73	0.62	0.26
GW44-09-1298	12/8/2008	Not applicable	Borehole	0.050	0.001	12.27	0.03	0.003	0.000	0.01	0.00	U	2.04	0.46	0.14
GW44-09-1299	12/8/2008	Not applicable	Borehole	0.065	0.000	14.70	0.03	0.002	0.000	0.01	0.00	U	2.18	0.49	0.02
GW44-09-1300	12/9/2008	Not applicable	Borehole	0.069	0.000	14.85	0.09	0.001	U	0.01	0.00	U	1.93	0.44	0.27
GW44-09-1301	12/9/2008	Not applicable	Borehole	0.097	0.000	15.48	0.07	0.001	U	0.01	0.00	U	2.00	0.45	0.31
GW44-09-1272	1/20/2009	Not applicable	Well, development	0.001	U	13.04	0.22	0.001	U	0.01	0.00	U	4.38	0.99	0.01
GW44-09-1273	1/20/2009	Not applicable	Well, development	0.001	U	11.61	0.15	0.001	U	0.01	0.00	U	4.48	1.01	0.01
GW44-09-1274	1/20/2009	Not applicable	Well, development	0.001	U	30.05	0.09	0.001	U	0.01	0.00	U	2.51	0.57	0.01
GW44-09-1275	1/20/2009	Not applicable	Well, development	0.001	U	23.86	0.29	0.001	U	0.01	0.00	U	2.54	0.57	0.01
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	0.001	U	9.74	0.05	0.001	U	0.01	0.00	U	4.99	1.13	0.01
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	0.001	U	9.43	0.07	0.001	U	0.01	0.00	U	4.97	1.12	0.01
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	0.001	U	9.39	0.08	0.001	U	0.01	0.00	U	5.04	1.14	0.01
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	0.001	U	9.45	0.11	0.001	U	0.01	0.00	U	5.04	1.14	0.01
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	0.001	U	9.04	0.02	0.001	U	0.01	0.00	U	5.00	1.13	0.01
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	0.001	U	8.65	0.05	0.001	U	0.01	0.00	U	5.08	1.15	0.01
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	0.001	0.000	15.27	0.02	0.002	0.000	0.01	0.00	U	2.67	0.60	0.01
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	0.001	0.000	13.99	0.18	0.002	0.000	0.01	0.00	U	2.69	0.61	0.01
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	0.001	U	12.63	0.05	0.001	U	0.01	0.00	U	2.69	0.61	0.01
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	0.001	U	12.42	0.14	0.001	U	0.01	0.00	U	2.70	0.61	0.01
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	0.001	U	11.62	0.12	0.001	U	0.01	0.00	U	2.66	0.60	0.01
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	0.001	U	11.46	0.04	0.002	0.000	0.01	0.00	U	2.73	0.62	0.01

 Table B-1.3.1-1

 Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	C2O4 (U)	Pb rslt (ppm)	stdev (Pb)	Lab pH	PO4(-3) rslt (ppm)	Rb rslt (ppm)	stdev (Rb)	Sb rslt (ppm)	stdev (Sb)	Se rslt (ppm)	stdev (Se)	Si rslt (ppm)
GW44-09-1292	11/17/2008	Not applicable	Borehole		0.0002	U	7.08	0.01, U	0.003	0.000	0.001	U	0.001	U	38.0
GW44-09-1293	12/8/2008	Not applicable	Borehole		0.0002	0.0000	7.90	0.03	0.002	0.000	0.001	U	0.001	U	22.9
GW44-09-1294	12/8/2008	Not applicable	Borehole		0.0002	U	8.03	0.03	0.002	0.000	0.001	U	0.001	U	21.7
GW44-09-1295	12/8/2008	Not applicable	Borehole		0.0002	U	7.66	0.07	0.001	U	0.001	U	0.001	U	12.0
GW44-09-1296	12/8/2008	Not applicable	Borehole		0.0009	0.0001	7.92	0.03	0.004	0.001	0.001	U	0.001	U	35.4
GW44-09-1297	12/8/2008	Not applicable	Borehole		0.0012	0.0002	7.93	0.07	0.003	0.000	0.001	U	0.001	U	32.4
GW44-09-1298	12/8/2008	Not applicable	Borehole		0.0007	0.0002	7.70	0.08	0.002	0.000	0.001	U	0.001	U	28.3
GW44-09-1299	12/8/2008	Not applicable	Borehole		0.0016	0.0000	7.70	0.05	0.006	0.000	0.001	U	0.001	U	30.1
GW44-09-1300	12/9/2008	Not applicable	Borehole		0.0002	U	8.25	0.03	0.001	U	0.001	U	0.001	U	19.8
GW44-09-1301	12/9/2008	Not applicable	Borehole		0.0002	U	7.95	0.17	0.001	U	0.001	U	0.001	U	20.4
GW44-09-1272	1/20/2009	Not applicable	Well, development	U	0.0002	U	7.57	0.01, U	0.002	0.000	0.001	U	0.001	U	32.5
GW44-09-1273	1/20/2009	Not applicable	Well, development	U	0.0002	U	7.56	0.01, U	0.002	0.000	0.001	U	0.001	U	30.2
GW44-09-1274	1/20/2009	Not applicable	Well, development	U	0.0002	U	7.83	0.08	0.002	0.000	0.001	U	0.001	U	37.1
GW44-09-1275	1/20/2009	Not applicable	Well, development	U	0.0002	U	7.82	0.01, U	0.002	0.000	0.001	U	0.001	U	35.9
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	U	0.0002	U	7.78	0.07	0.002	0.000	0.001	U	0.001	U	33.1
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	U	0.0002	U	7.62	0.08	0.002	0.000	0.001	U	0.001	U	32.8
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	U	0.0002	U	7.66	0.06	0.002	0.000	0.001	U	0.001	U	33.3
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	U	0.0002	U	7.75	0.08	0.002	0.000	0.001	U	0.001	U	33.9
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	U	0.0002	U	7.76	0.08	0.002	0.000	0.001	U	0.001	U	32.8
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	U	0.0002	U	7.78	0.09	0.002	0.000	0.001	U	0.001	U	32.0
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	U	0.0002	U	7.75	0.04	0.002	0.000	0.001	U	0.001	U	33.8
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	U	0.0002	U	7.75	0.06	0.002	0.000	0.001	U	0.001	U	34.9
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	U	0.0002	U	7.79	0.07	0.002	0.000	0.001	U	0.001	U	34.6
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	U	0.0002	U	7.80	0.06	0.002	0.000	0.001	U	0.001	U	35.0
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	U	0.0002	U	7.79	0.07	0.002	0.000	0.001	U	0.001	U	34.4
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	U	0.0002	U	7.81	0.06	0.002	0.000	0.001	U	0.001	U	34.8
Table B-1.3.1-1

 Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	stdev (Si)	SiO2 rslt (ppm)	stdev (SiO2)	Sn rslt (ppm)	stdev (Sn)	SO4(-2) rslt (ppm)	Sr rslt (ppm)	stdev (Sr)	Th rslt (ppm)	stdev (Th)	Ti rslt (ppm)	stdev (Ti)
GW44-09-1292	11/17/2008	Not applicable	Borehole	0.4	81.3	0.9	0.001	U	4.93	0.062	0.001	0.001	U	0.016	0.000
GW44-09-1293	12/8/2008	Not applicable	Borehole	0.1	49.0	0.3	0.001	U	6.63	0.067	0.001	0.001	U	0.032	0.000
GW44-09-1294	12/8/2008	Not applicable	Borehole	0.4	46.5	0.8	0.001	U	7.66	0.061	0.001	0.001	U	0.014	0.001
GW44-09-1295	12/8/2008	Not applicable	Borehole	0.1	25.6	0.2	0.001	U	4.46	0.042	0.000	0.001	U	0.009	0.000
GW44-09-1296	12/8/2008	Not applicable	Borehole	0.0	75.7	0.1	0.001	U	6.65	0.052	0.000	0.001	U	0.038	0.000
GW44-09-1297	12/8/2008	Not applicable	Borehole	0.1	69.3	0.3	0.001	U	3.60	0.048	0.000	0.001	U	0.052	0.000
GW44-09-1298	12/8/2008	Not applicable	Borehole	0.2	60.6	0.4	0.001	U	2.88	0.044	0.000	0.001	U	0.031	0.001
GW44-09-1299	12/8/2008	Not applicable	Borehole	0.6	64.4	1.2	0.001	U	3.70	0.053	0.000	0.001	U	0.056	0.000
GW44-09-1300	12/9/2008	Not applicable	Borehole	0.1	42.4	0.3	0.001	U	4.90	0.081	0.001	0.001	U	0.002	U
GW44-09-1301	12/9/2008	Not applicable	Borehole	0.2	43.7	0.4	0.001	U	3.28	0.036	0.000	0.001	U	0.007	0.000
GW44-09-1272	1/20/2009	Not applicable	Well, development	0.4	69.6	0.9	0.001	U	6.07	0.058	0.001	0.001	U	0.002	U
GW44-09-1273	1/20/2009	Not applicable	Well, development	0.5	64.6	1.1	0.001	U	5.83	0.053	0.001	0.001	U	0.002	U
GW44-09-1274	1/20/2009	Not applicable	Well, development	0.4	79.4	0.8	0.001	U	13.8	0.089	0.000	0.001	U	0.003	0.000
GW44-09-1275	1/20/2009	Not applicable	Well, development	0.6	76.7	1.2	0.001	U	11.5	0.083	0.001	0.001	U	0.002	U
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	0.4	70.8	0.8	0.001	U	4.44	0.052	0.000	0.001	U	0.002	U
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	0.2	70.3	0.3	0.001	U	4.31	0.050	0.000	0.001	U	0.002	U
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	0.2	71.2	0.5	0.001	U	4.33	0.051	0.001	0.001	U	0.002	U
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	0.3	72.6	0.7	0.001	U	4.45	0.052	0.001	0.001	U	0.002	U
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	0.1	70.2	0.2	0.001	U	4.20	0.050	0.000	0.001	U	0.002	U
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	0.2	68.4	0.4	0.001	U	4.25	0.048	0.000	0.001	U	0.002	U
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	0.1	72.4	0.3	0.001	U	7.38	0.065	0.001	0.001	U	0.002	U
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	0.2	74.8	0.5	0.001	U	6.32	0.063	0.000	0.001	U	0.002	U
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	0.2	74.0	0.4	0.001	U	5.74	0.062	0.000	0.001	U	0.002	U
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	0.4	74.9	0.8	0.001	U	5.25	0.061	0.000	0.001	U	0.002	U
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	0.3	73.5	0.7	0.001	U	4.79	0.059	0.001	0.001	U	0.002	U
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	0.1	74.6	0.3	0.001	U	4.71	0.058	0.000	0.001	U	0.002	U

Notes: U = Not detected. Total organic carbon not analyzed in borehole samples collected in high-dens

 Table B-1.3.1-1

 Analytical Results for Groundwater-Screening Samples Collected from Well R-44, Mortandad Canyon

Sample ID	Date Received	Time	Sample Type	TI rslt (ppm)	stdev (TI)	U rslt (ppm)	stdev (U)	V rslt (ppm)	stdev (V)	Zn rslt (ppm)	stdev (Zn)	TDS (ppm)	Cations	Anions	Balance
GW44-09-1292	11/17/2008	Not applicable	Borehole	0.001	U	0.0003	0.0000	0.004	0.000	0.003	0.000	272.0	2.28	2.50	-0.05
GW44-09-1293	12/8/2008	Not applicable	Borehole	0.001	U	0.0011	0.0001	0.002	0.000	0.046	0.000	196.7	1.91	1.93	-0.01
GW44-09-1294	12/8/2008	Not applicable	Borehole	0.001	U	0.0013	0.0000	0.002	0.000	0.026	0.000	207.8	1.96	2.15	-0.05
GW44-09-1295	12/8/2008	Not applicable	Borehole	0.001	U	0.0003	0.0000	0.002	0.000	0.037	0.000	143.1	1.47	1.54	-0.03
GW44-09-1296	12/8/2008	Not applicable	Borehole	0.001	U	0.0024	0.0003	0.006	0.001	0.041	0.002	218.0	1.62	1.95	-0.09
GW44-09-1297	12/8/2008	Not applicable	Borehole	0.001	U	0.0014	0.0002	0.005	0.001	0.047	0.002	206.7	1.67	1.80	-0.04
GW44-09-1298	12/8/2008	Not applicable	Borehole	0.001	U	0.0005	0.0001	0.004	0.001	0.052	0.001	185.1	1.50	1.59	-0.03
GW44-09-1299	12/8/2008	Not applicable	Borehole	0.001	U	0.0010	0.0000	0.003	0.000	0.017	0.002	203.3	1.69	1.79	-0.03
GW44-09-1300	12/9/2008	Not applicable	Borehole	0.001	U	0.0034	0.0001	0.006	0.000	0.020	0.000	244.7	2.32	2.78	-0.09
GW44-09-1301	12/9/2008	Not applicable	Borehole	0.001	U	0.0007	0.0000	0.002	0.000	0.009	0.000	168.6	1.45	1.66	-0.07
GW44-09-1272	1/20/2009	Not applicable	Well, development	0.001	U	0.0008	0.0000	0.005	0.000	0.004	0.001	202.0	1.55	1.77	-0.07
GW44-09-1273	1/20/2009	Not applicable	Well, development	0.001	U	0.0008	0.0000	0.004	0.000	0.003	0.001	193.0	1.46	1.74	-0.09
GW44-09-1274	1/20/2009	Not applicable	Well, development	0.001	U	0.0019	0.0000	0.007	0.000	0.006	0.001	267.0	2.32	2.46	-0.03
GW44-09-1275	1/20/2009	Not applicable	Well, development	0.001	U	0.0016	0.0000	0.007	0.000	0.007	0.002	250.0	2.08	2.29	-0.05
GW44-09-1276	2/16/2009	12:00:00 PM	Well, pumping test	0.001	U	0.0005	0.0000	0.005	0.000	0.013	0.001	193.0	1.32	1.66	-0.11
GW44-09-1277	2/16/2009	2:00:00 PM	Well, pumping test	0.001	U	0.0005	0.0000	0.005	0.000	0.010	0.001	188.0	1.29	1.59	-0.10
GW44-09-1278	2/16/2009	8:00:00 PM	Well, pumping test	0.001	U	0.0005	0.0000	0.005	0.000	0.009	0.001	188.0	1.31	1.58	-0.10
GW44-09-1279	2/17/2009	12:00:00 AM	Well, pumping test	0.001	U	0.0005	0.0000	0.006	0.000	0.008	0.002	190.0	1.33	1.59	-0.09
GW44-09-1280	2/17/2009	4:00:00 AM	Well, pumping test	0.001	U	0.0005	0.0000	0.005	0.000	0.007	0.002	190.0	1.28	1.63	-0.12
GW44-09-1281	2/17/2009	8:00:00 AM	Well, pumping test	0.001	U	0.0005	0.0000	0.005	0.000	0.005	0.000	186.0	1.26	1.60	-0.12
GW44-09-1282	2/21/2009	12:00:00 PM	Well, pumping test	0.001	U	0.0009	0.0000	0.007	0.000	0.006	0.001	212.0	1.65	1.85	-0.06
GW44-09-1283	2/21/2009	4:00:00 PM	Well, pumping test	0.001	U	0.0008	0.0000	0.006	0.000	0.006	0.000	210.0	1.62	1.79	-0.05
GW44-09-1284	2/21/2009	8:00:00 PM	Well, pumping test	0.001	U	0.0008	0.0000	0.006	0.000	0.005	0.001	205.0	1.57	1.74	-0.05
GW44-09-1285	2/22/2009	12:00:00 AM	Well, pumping test	0.001	U	0.0008	0.0000	0.007	0.000	0.006	0.001	206.0	1.57	1.73	-0.05
GW44-09-1286	2/22/2009	4:00:00 AM	Well, pumping test	0.001	U	0.0007	0.0000	0.007	0.000	0.005	0.002	202.0	1.53	1.70	-0.05
GW44-09-1287	2/22/2009	8:00:00 AM	Well, pumping test	0.001	U	0.0007	0.0000	0.006	0.000	0.006	0.001	203.0	1.53	1.69	-0.05

Notes: U = Not detected. Total organic carbon not analyzed in borehole samples collected in high-dens

R-44 Well Completion Report

Appendix C

Aquifer Testing Report

C-1.0 INTRODUCTION

This appendix describes the hydraulic analysis of pumping tests at well R-44 screens 1 and 2 located in Mortandad Canyon near the edge of the existing chromium plume beneath the canyon. The tests were conducted in conjunction with testing of nearby well R-45 screens 1 and 2. The primary objective of the analysis was to determine the hydraulic properties of the zones screened in R-44, as well as the intervening sediments between the two screen zones. A secondary objective was to look for cross-connection between R-44 and surrounding wells R-45, R-11, R-13, and R-28.

Testing consisted primarily of constant-rate pumping tests conducted on R-44 screens 1 and 2. During the tests on each screen, water levels were monitored in the nonpumped screen zone in R-44 to examine the properties of the intervening sediments, and in R-45 screens 1 and 2 to monitor cross-connection between the wells. In addition, water levels were monitored in adjacent wells R-11, R-13, and R-28.

Consistent with most of the R-well pumping tests conducted on the plateau, an inflatable packer system was used in R-44 to isolate the screens and eliminate the effects of casing storage on the test data.

Conceptual Hydrogeology

R-44 is a dual-screen well completed in the Puye Formation just above the Miocene pumiceous deposits, with 10 ft of screen from 895.0 to 905.0 ft below ground surface (bgs) (screen 1)] and 9.9 ft of screen from 985.3 to 995.2 ft bgs (screen 2); the screens are separated by 80.3 ft of intervening sediments. The composite static water level measured on February 13 at the onset of testing was 878.86 ft bgs. When the zones were isolated with inflatable packers, the water level in screen 1 rose 0.06 ft to 878.80 ft bgs, while the level in screen 2 dropped 0.14 ft to 879.00 ft bgs. Thus, the initial water level in screen 1 was 0.2 ft higher than that in screen 2, implying a downward gradient. The head difference between the two screen zones in R-44 was modest (0.0022 ft downward gradient from the center of screen 1 to the center of screen 2) compared with differences measured at other multiscreen wells on the plateau, which show head differences of feet or tens of feet in most cases. The brass cap elevation at R-44 is 6714.91 ft above mean sea level (amsl), making the approximate static water-level elevations in screens 1 and 2 5836 ft.

Well R-45, also a dual-screen well, is located about 1000 ft north of R-44 and is completed at the top of the regional aquifer with the upper screen in the Puye Formation and the lower screen in the Miocene pumiceous sediments. Screen 1 is 10 ft long, set between 880 and 890 ft bgs. Screen 2 is 20 ft long, extending from 974.9 to 994.9 ft bgs. The composite water level in R-45 measured at the outset of testing R-44 and R-45 was 868.27 ft bgs. When the zones were isolated with inflatable packers, the water level in screen 1 rose from 0.04 to 868.23 ft bgs, while the level in screen 2 dropped 0.07 to 868.34 ft bgs. Thus, the initial water level in screen 1 was just 0.11 ft above that in screen 2. The brass cap elevation at R-44 is 6704.02 ft amsl, making the approximate static water-level elevations 5836 ft in screens 1 and 2.

R-44 Screen 1 Testing

R-44 screen 1 was tested from February 14 to February 18, 2009. Testing consisted of brief trial pumping on February 14, background data collection, and a 24-h constant-rate pumping test that was begun on February 16.

Two trial tests were conducted on February 14. Trial 1 was conducted at an average discharge rate of 19.2 gpm for 60 min from 8:00 to 9:00 a.m. (all times Mountain Standard Time) and was followed by 60 min of recovery until 10:00 a.m. Trial 2 was conducted for 120 min from 10:00 a.m. to 12:00 p.m. at 20.0 gpm. Following shutdown, recovery/background was monitored for 44 h until 8:00 a.m. on February 16.

During the trial tests, the generator supplying power to the submersible pump operated erratically with fluctuating voltage and alternating current frequency as well as substandard current frequency. This caused undesirable fluctuations in the discharge rate and limited the maximum rate that could be obtained. On February 15, a replacement generator was installed and run for about 10 mi from 11:34 to 11:44 a.m. to verify operation and rotation direction on the pump.

At 8:00 a.m. on February 16, the 24-h pumping test was begun at a rate of 24.2 gpm. Pumping continued until 8:00 a.m. on February 17. Following shutdown, recovery measurements were recorded for 24 h until 8:00 a.m. on February 18.

R-44 Screen 2 Testing

R-44 screen 2 was tested from February 19 to February 23, 2009. Testing consisted of brief trial pumping on February 19, background data collection, and a 24-h constant-rate pumping test that was begun on February 21.

Two trial tests were conducted on February 19. Trial 1 was conducted at a discharge rate of 23.9 gpm for 60 min from 8:00 to 9:00 a.m. and was followed by 60 min of recovery until 10:00 a.m. Trial 2 was conducted for 120 min from 10:00 a.m. to 12:00 p.m. at 24.0 gpm. Following shutdown, recovery/ background was monitored for 44 h until 8:00 a.m. on February 21.

At 8:00 a.m. on February 21, the 24-h pumping test was begun at a rate of 23.9 gpm. Pumping continued until 8:00 a.m. on February 22. Following shutdown, recovery measurements were recorded for 24 h until 8:00 a.m. on February 23.

Leaky Drop Pipe Joints

During the R-44 testing, there was leakage through the threaded joints on the 1 $\frac{1}{2}$ -in. stainless-steel drop pipe (1.90-in. outside diameter [O.D.] × 1.61-in. inside diameter [I.D.]), creating downhole voids inside the drop pipe beneath the check valves. This allowed initial pump operation against reduced head until the voids were refilled. The result was an elevated pumping rate for a brief period at the beginning of most of the tests. This effect corrupted the early startup data and added uncertainty to the analyses of the early drawdown data. The leaks were caused by either worn or improperly manufactured threads, as well as the need to avoid wrenching the pipe extremely as a precaution against galling the stainless-steel threads.

C-2.0 BACKGROUND DATA

The background water-level data collected in conjunction with running the pumping tests allow the analyst to see what water-level fluctuations occur naturally in the aquifer and help distinguish between water-level changes caused by conducting the pumping test and changes associated with other causes.

Background water-level fluctuations have several causes, among them barometric pressure changes, operation of other wells in the aquifer, Earth tides, and long-term trends related to weather patterns. The background data hydrographs from the monitored wells were compared with barometric pressure data from the area to determine if a correlation existed.

Previous pumping tests on the plateau have demonstrated a barometric efficiency for most wells of between 90% and 100%. Barometric efficiency is defined as the ratio of water-level change divided by barometric pressure change, expressed as a percentage. In the initial pumping tests conducted on the early R-wells, downhole pressure was monitored using a vented pressure transducer. This equipment measures the difference between the total pressure applied to the transducer and the barometric pressure, this difference being the true height of water above the transducer.

Subsequent pumping tests, including R-44, have utilized nonvented transducers. These devices simply record the total pressure on the transducer, that is, the sum of the water height plus the barometric pressure. This results in an attenuated "apparent" hydrograph in a barometrically efficient well. Take as an example a 90% barometrically efficient well. When monitored using a vented transducer, an increase in barometric pressure of 1 unit causes a decrease in recorded downhole pressure of 0.9 unit because the water level is forced downward 0.9 unit by the barometric pressure change. However, using a nonvented transducer, the total measured pressure increases by 0.1 unit (the combination of the barometric pressure increase and the water-level decrease). Thus, the resulting apparent hydrograph changes by a factor of 100 minus the barometric efficiency, and in the same direction as the barometric pressure change, rather than in the opposite direction.

Barometric pressure data were obtained from Technical Area 54 (TA-54) tower site from the Waste and Environmental Services Division-Environmental Data and Analysis (WES-EDA). The TA-54 measurement location is at an elevation of 6548 ft amsl, whereas the wellhead elevation is approximately 6715 ft amsl. The static water levels of the two zones were about 879 ft below land surface, making the water-table elevation roughly 5836 ft amsl. Therefore, the measured barometric pressure data from TA-54 had to be adjusted to reflect the pressure at the elevation of the water table within R-44.

The following formula was used to adjust the measured barometric pressure data:

$$P_{WT} = P_{TA54} \exp\left[-\frac{g}{3.281R}\left(\frac{E_{R44} - E_{TA54}}{T_{TA54}} + \frac{E_{WT} - E_{R44}}{T_{WELL}}\right)\right]$$
Equation C-1

Where, P_{WT} = barometric pressure at the water table inside R-44

P_{TA54} = barometric pressure measured at TA-54

g = acceleration of gravity, in m/sec² (9.80665 m/sec²)

R = gas constant, in J/Kg/degree Kelvin (287.04 J/Kg/degree Kelvin)

 E_{R44} = land surface elevation at R-44 site, in feet (6715 ft)

E_{7A54} = elevation of barometric pressure measuring point at TA-54, in feet (6548 ft)

 E_{WT} = elevation of the water level in R-44, in feet (approximately 5836 ft)

 T_{TA54} = air temperature near TA-54, in degrees Kelvin (assigned a value of 34.2 degrees Fahrenheit, or 284.4 degrees Kelvin)

 T_{WELL} = air temperature inside R-44, in degrees Kelvin (assigned a value of 62.1 degrees Fahrenheit, or 289.9 degrees Kelvin)

This formula is an adaptation of an equation WES-EDA provided. It can be derived from the ideal gas law and standard physics principles. An inherent assumption in the derivation of the equation is that the air temperature between TA-54 and the well is temporally and spatially constant, and that the temperature of the air column in the well is similarly constant.

The corrected barometric pressure data reflecting pressure conditions at the water table were compared with the water-level hydrographs to discern the correlation between the two.

C-3.0 IMPORTANCE OF EARLY DATA

When pumping or recovery first begins, the vertical extent of the cone of depression is limited to approximately the well screen length, the filter pack length or, the aquifer thickness in relatively thin permeable strata. For many pumping tests on the plateau, the early pumping period is the only time that the effective height of the cone of depression is known with certainty. Thus, the early data often offer the best opportunity to obtain hydraulic conductivity information because conductivity would equal the earliest-time transmissivity divided by the well screen length.

Unfortunately, in many pumping tests, casing-storage effects dominate the early-time data, hindering the effort to determine the transmissivity of the screened interval. The duration of casing-storage effects can be estimated using the following equation (Schafer 1978, 098240).

$$t_c = \frac{0.6(D^2 - d^2)}{\frac{Q}{s}}$$

Equation C-2

Where, t_c = duration of casing storage effect, in minutes

- D = inside diameter of well casing, in inches
- d = outside diameter of column pipe, in inches
- Q = discharge rate, in gallons per minute
- s = drawdown observed in pumped well at time t_c , in feet

In some instances, it is possible to eliminate casing storage effects by setting an inflatable packer above the tested screen interval before conducting the test. Therefore, this option has been implemented for the R-well testing program, including the R-44 pumping tests.

C-4.0 TIME-DRAWDOWN METHODS

Time-drawdown data can be analyzed using a variety of methods. Among them is the Theis method (1934-1935, 098241). The Theis equation describes drawdown around a well as follows:

 $s = \frac{114.6Q}{T}W(u)$

Where,

$$W(u) = \int_{u}^{\infty} \frac{e^{-x}}{x} dx$$
 Equation C-4

and

$$u = \frac{1.87r^2S}{Tt}$$
 Equation C-5

Equation C-3

and where, s = drawdown, in feet

- Q = discharge rate, in gallons per minute
- T = transmissivity, in gallons per day per foot
- S = storage coefficient (dimensionless)
- t = pumping time, in days
- r = distance from center of pumpage, in feet

To use the Theis method of analysis, the time-drawdown data are plotted on log-log graph paper. Then, Theis curve matching is performed using the Theis type curve—a plot of the Theis well function W(u) versus 1/u. Curve matching is accomplished by overlaying the type curve on the data plot and, while keeping the coordinate axes of the two plots parallel, shifting the data plot to align with the type curve, effecting a match position. An arbitrary point, referred to as the match point, is selected from the overlapping parts of the plots. Match-point coordinates are recorded from the two graphs, yielding four values:W(u): 1/u, s, and t. Using these match-point values, transmissivity and storage coefficient are computed as follows:

$$T = \frac{114.6Q}{s} W(u)$$
Equation C-6
$$S = \frac{Tut}{2693r^2}$$
Equation C-7

Where, T = transmissivity, in gallons per day per foot

S = storage coefficient

Q = discharge rate, in gallons per minute

W(u) = match-point value

- s = match-point value, in feet
- u = match-point value
- t = match-point value, in minutes

An alternative solution method applicable to time-drawdown data is the Cooper–Jacob method (1946, 098236), a simplification of the Theis equation that is mathematically equivalent to the Theis equation for most pumped well data. The Cooper–Jacob equation describes drawdown around a pumping well as follows:

$$s = \frac{264Q}{T} \log \frac{0.3Tt}{r^2 S}$$
 Equation C-8

The Cooper–Jacob equation is a simplified approximation of the Theis equation and is valid whenever the u value is less than about 0.05. For small radius values (e.g., corresponding to borehole radii), u is less than 0.05 at very early pumping times and therefore is less than 0.05 for most or all measured drawdown values. Thus, for the pumped well, the Cooper–Jacob equation usually can be considered a valid approximation of the Theis equation.

According to the Cooper–Jacob method, the time-drawdown data are plotted on a semilog graph, with time plotted on the logarithmic scale. Then a straight line of best fit is constructed through the data points and transmissivity is calculated using:

$$T = \frac{264Q}{\Delta s}$$

Equation C-9

Where, T = transmissivity, in gallons per day per foot

- Q = discharge rate, in gallons per minute
- Δs = change in head over one log cycle of the graph, in feet

Because the R-wells are severely partially penetrating, an alternate solution considered for assessing aquifer conditions is the Hantush equation for partially penetrating wells (Hantush 1961, 098237; Hantush 1961, 106003). The Hantush equation is as follows:

Equation C-10

$$s = \frac{Q}{4\pi T} \left[W(u) + \frac{2b^2}{\pi^2 (l-d)(l'-d')} \sum_{n=1}^{\infty} \frac{1}{n^2} \left(\sin \frac{n\pi d}{b} - \sin \frac{n\pi d}{b} \right) \left(\sin \frac{n\pi d'}{b} - \sin \frac{n\pi d'}{b} \right) W\left(u, \sqrt{\frac{K_z}{K_r}} \frac{n\pi r}{b} \right) \right]$$

Where, in consistent units, s, Q, T, t, r, S, and u are as previously defined and

- b = aquifer thickness
- d = distance from top of aquifer to top of well screen in pumped well
- / = distance from top of aquifer to bottom of well screen in pumped well
- d' = distance from top of aquifer to top of well screen in observation well
- l' = distance from top of aquifer to bottom of well screen in observation well
- K_z = vertical hydraulic conductivity
- K_r = horizontal hydraulic conductivity

In this equation, W(u) is the Theis well function and $W(u,\beta)$ is the Hantush well function for leaky aquifers where:

$$\beta = \sqrt{\frac{K_z}{K_r}} \frac{n\pi r}{b}$$
 Equation C-11

Note that for single-well tests, d = d' and l = l'.

C-5.0 RECOVERY METHODS

Recovery data were analyzed using the Theis recovery method. This is a semilog analysis method similar to the Cooper–Jacob procedure.

In this method, residual drawdown is plotted on a semilog graph versus the ratio t/t', where *t* is the time since pumping began and *t'* is the time since pumping stopped. A straight line of best fit is constructed through the data points and *T* is calculated from the slope of the line as follows:

$$T = \frac{264Q}{\Delta s}$$
 Equation C-12

The recovery data are particularly useful compared with time-drawdown data. Because the pump is not running, spurious data responses associated with dynamic discharge rate fluctuations are eliminated. The result is that the data set is generally "smoother" and easier to analyze. This was of paramount importance in the R-44 pumping tests because of the entrained air induced discharge rate fluctuations.

C-6.0 SPECIFIC CAPACITY METHOD

The specific capacity of the pumped well can be used to obtain a lower-bound value of hydraulic conductivity. The hydraulic conductivity is computed using formulas that are based on the assumption that the pumped well is 100% efficient. The resulting hydraulic conductivity is the value required to sustain the observed specific capacity. If the actual well is less than 100% efficient, it follows that the actual hydraulic conductivity would have to be greater than calculated to compensate for well inefficiency. Thus, because the efficiency is unknown, the computed hydraulic conductivity value represents a lower bound. The actual conductivity is known to be greater than or equal to the computed value.

For fully penetrating wells, the Cooper–Jacob equation can be iterated to solve for the lower-bound hydraulic conductivity. However, the Cooper–Jacob equation (assuming full penetration) ignores the contribution to well yield from permeable sediments above and below the screened interval. To account for this contribution, it is necessary to use a computation algorithm that includes the effects of partial penetration. One such approach was introduced by Brons and Marting (1961, 098235) and augmented by Bradbury and Rothchild (1985, 098234).

Brons and Marting introduced a dimensionless drawdown correction factor, s_P, approximated by Bradbury and Rothschild as follows:

$$s_{p} = \frac{1 - \frac{L}{b}}{\frac{L}{b}} \left[\ln \frac{b}{r_{w}} - 2.948 + 7.363 \frac{L}{b} - 11.447 \left(\frac{L}{b}\right)^{2} + 4.675 \left(\frac{L}{b}\right)^{3} \right]$$
 Equation C-13

In this equation, L is the well screen length, in ft. Incorporating the dimensionless drawdown parameter, the conductivity is obtained by iterating the following formula:

$$K = \frac{264Q}{sb} \left(\log \frac{0.3Tt}{r_w^2 S} + \frac{2s_P}{\ln 10} \right)$$
 Equation C-14

To apply this procedure, a storage coefficient value must be assigned. Unconfined conditions were assumed for screen 1, while confined to leaky-confined conditions were applied to screen 2. Storage coefficient values for confined conditions can be expected to range from about 10^{-5} to 10^{-3} , depending on aquifer thickness, while those for unconfined conditions can be expected to range from about 0.01 to 0.25 (Driscoll 1986, 104226). The calculation result is not particularly sensitive to the choice of storage coefficient value, so a rough estimate of the storage coefficient is generally adequate to support the calculations. An assumed value of 0.1 was used in the calculations for screen 1, while values of 10^{-3} and 10^{-2} were used for screen 2. For screen 2, a storage coefficient value of 10^{-3} was deemed appropriate for the assumption of confined conditions (with perhaps very minor leakage from above), while 10^{-2} was used to simulate leaky-confined conditions.

The analysis also requires assigning a value for the saturated aquifer thickness, b. For calculation purposes, the screen 1 zone was assumed to extend from the water table, at 879 ft bgs, to the midpoint of the blank pipe section between the two screens, at approximately 945 ft bgs. This resulted in an assigned aquifer thickness of 67 ft for screen 1. This was equivalent to assuming that the resistive zone between screens 1 and 2 was at the midpoint of the intervening blank section, even though the actual location of the aquitard was not known. However, the computed result is not particularly sensitive to the exact aquifer thickness, because sediments far above or below the screen have little effect on yield and drawdown response. Therefore, the calculation based on the assumed aquifer thickness value was deemed to be adequate. For screen 2, an arbitrary thickness of 200 ft was assigned in the calculations.

Computing the lower-bound estimate of hydraulic conductivity can provide a useful frame of reference for evaluating the other pumping test calculations.

C-7.0 BACKGROUND DATA ANALYSIS

Background aquifer pressure data collected during the R-44 tests were plotted along with barometric pressure to determine the barometric effect on water levels and to look for pumping response in the surrounding observation wells. The four screen zones in R-44 and R-45 were monitored using nonvented pressure transducers, while the remaining wells—R-11, R-13, and R-28—were monitored using vented transducers.

Figure C-7.0-1 shows aquifer pressure data from R-44 screen 1 along with barometric pressure data from TA-54 that have been corrected to equivalent barometric pressure in feet of water at the water table. The R-44 data are referred to in the figure as the "apparent hydrograph" because the measurements reflect the sum of water pressure and barometric pressure, having been recorded using a nonvented pressure transducer. The times of the pumping periods for the screen 1 and screen 2 pumping tests are included on the figure for reference.

The transducers used in screens 1 and 2 were switched between tests, accounting for the different appearance in the data output from the screen 1 tests to the screen 2 tests. The transducer used initially (during the screen 1 test) showed substantial scatter, giving the thick-appearing plot of data points. The second transducer (right side of graph) showed less scatter, except during the pumping periods when significant scatter was observed. This resulted from the transducer having to be located adjacent to the pump power cable (inevitable when pumping screen 2 and monitoring screen 1), which interfered with transducer operation when the pump was running. The second transducer showed some sort of a dry problem (oil-canning) as indicated by the "striped" or "layered" effect seen in the data trace on February 19, 22 and 23. This effect had been seen previously during the testing of R-16r in 2005 and is believed to indicate a transducer malfunction of some sort.

To minimize the data scatter on Figure C-7.0-1, a rolling average of the data was plotted in Figure C-7.0-2. The average included data over a 1-h interval.

It appeared in Figures C-7.0-1 and C-7.0-2 that changes in barometric pressure had no discernible effect on water levels. An example illustrating this was the abrupt drop and subsequent rise in barometric pressure on February 20 that appeared to have no corresponding effect on the total aquifer pressure.

As a check on this, a plot was made of background data collected subsequently from R-44 screen 1 during the R-45 pumping tests conducted in late February and early March. Figure C-7.0-3 shows the observed apparent hydrograph and the corresponding barometric pressure. Figure C-7.0-3 confirmed that changes in barometric pressure had no effect on the aquifer pressure. The clincher was the tremendous change in barometric pressure that occurred from February 27 to 28 with no corresponding perturbation in the apparent hydrograph. This implied a high barometric efficiency for screen 1, essentially 100%.

Aside from the lack of response to barometric pressure changes, there were two other key observations made from the data shown in Figures C-7.0-1 and C-7.0-2. First, during the background data collection before the screen 1 pumping test, there was a distinct, steady decline in aquifer pressure totaling about 0.03 ft over 2.5 d. It was believed that this was a response to operation of Los Alamos County well PM-4, which began pumping on February 11 and ran continuously until March 4. The other supply wells cycled randomly throughout this period (illustrated below) and would not have caused the observed effect.

Second, there was a distinct response in screen 1 to pumping screen 2—both during the trial tests on February 19 and the 24-h test on February 21. During the 24-h screen 2 pumping period, the observed drawdown in screen 1 was about 0.05 ft. Following pump shutoff, there was a slow recovery effect, typical of the response of distant observation wells or wells separated from the pumped zone by an aquitard.

Figure C-7.0-4 shows the apparent hydrograph for R-44 screen 2 recorded during the screen 1 and 2 test periods. The times of the screen 1 and 2 pumping tests are included in the figure for reference. Again, the transducers were switched between tests, accounting for the difference in the appearance of the data plots from one test to the other. Note that the transducer used during the screen 2 test (right side of Figure C-7.0-4) was the same one that was used to monitor screen 1 during the screen 1 pumping test (left side of Figure C-7.0-1). The broad data scatter was consistent in both plots and apparently unique to that particular transducer.

To remove some of the scatter in the plot, a rolling average of the data was prepared as shown in Figure C-7.0-5.

Finally, an additional plot was prepared in Figure C-7.0-6 comparing the aquifer pressure response with the times of operation of Los Alamos County production wells PM-3, PM-5, and O-4. PM-4 was not included in the plot, as it operated continuously throughout the time period shown on the graph.

The data from Figures C-7.0-4, C-7.0-5, and C-7.0-6 were examined to discern the relationships between aquifer pressure and both barometric pressure fluctuations and municipal pumping. There were some hints of a possible correlation of aquifer pressure and changes in barometric pressure. For example, a decline in barometric pressure on February 16 seemed to coincide with a drop in aquifer pressure, while rises in barometric pressure late on February 18 and 20 matched increases in aquifer pressure. The decline in aquifer pressure on February 16 occurred during the 24-h pumping test, so it may have been a response to pumping. However, there was no such analogous explanation for the aquifer pressure increases.

To provide further insight into the relationship between screen 2 water levels and barometric pressure, a plot was made of background data collected subsequently from R-44 screen 2 during the R-45 pumping tests conducted in late February and early March. Figure C-7.0-7 shows the observed apparent

hydrograph and the corresponding barometric pressure. Because of the scatter in the data set, a rolling average plot was prepared as shown in Figure C-7.0-8.

The data shown in Figures C-7.0-7 and C-7.0-8 showed that barometric pressure changes, in fact, caused no change in aquifer pressure. This was best illustrated by the observations made from February 27 to 28. The tremendous rise in barometric pressure during this period had no effect on aquifer pressure. This implied essentially a 100% barometric efficiency for screen 2, similar to what was observed for screen 1. This meant that the aquifer pressure increases seen on February 18 and 20, as well as the decline observed on February 16, were not attributable to barometric pressure fluctuations. (The diurnal fluctuations having a magnitude of about 0.03 ft in Figures C-7.0-7 and C-7.0-8 were responses to Earth tides.)

The data in Figures C-7.0-4 and C-7.0-5 were reexamined in light of knowing that barometric pressure fluctuations did not affect the apparent hydrograph. The background data leading up to the screen 1 24-h pumping test (February 13 to 16) showed a steady pressure decline of about 0.06 ft in 2.5 d. This was likely attributable to operation of PM-4, which was started on February 11 and run continuously. The response to pumping PM-4 in screen 2 was twice as great as that in screen 1 (0.06 ft versus 0.03 ft). This implied the possibility of a zone of limited permeability separating screens 1 and 2, effectively providing greater hydraulic isolation of screen 1 from the effects of PM-4 operation. This was also consistent with the minimal drawdown observed in each screen (0.05 ft) due to pumping the other screen.

During the 24-h pumping test on screen 1, the rate of water-level decline in screen 2 increased, indicating a response to the pumping test. Following pump shutoff, there was a slow recovery, typical of the response of distant observation wells or wells separated from the pumped zone by an aquitard. A rough estimate of the drawdown induced in screen 2 by pumping screen 1 was 0.05 ft.

An examination of the production well operation schedule in Figure C-7.0-6 showed no correlation between screen 2 aquifer pressure and cycling of production wells PM-3, PM-5, and O-4. There was no obvious explanation for the aquifer pressure increases observed on February 18 and 20 and no such response was observed in R-44 screen 1. It is possible that these fluctuations may have been attributable to Earth tides.

Figure C-7.0-9 shows the apparent hydrograph for R-45 screen 1. The transducer output showed the same bizarre striped/layered effect that was observed from one of the transducers used to monitor R-44. There appeared to be no aquifer pressure response to changes in barometric pressure, implying a barometric efficiency of essentially 100%.

The gradual decline in pressure from February 13 to February 19 likely was caused by the startup and continuous operation of production well PM-4 beginning on February 11. The aquifer pressure declined approximately 0.11 ft over a 6-d period.

There was no discernible response in R-45 screen 1 to test pumping R-44 screen 1, although the unusual transducer output may have masked subtle changes in water level. There appeared to be a response, however, to pumping R-44 screen 2. The water level in R-45 screen 1 dropped roughly 0.02 ft during the 24-h pumping test in R-44 screen 2.

The data in Figure C-7.0-9 were replotted in Figure C-7.0-10 along with operating times for production wells PM-3, PM-5 and O-4. Examination of the data showed that there was no discernible response in R-45 screen 1 to cycling these three production wells.

Figure C-7.0-11 shows the apparent hydrograph for R-45 screen 2. To eliminate some of the data scatter, a rolling average plot was prepared also as shown in Figure C-7.0-12. Several observations can be made from these graphs.

The aquifer pressure declined for a few days before the R-44 screen 1 pumping test and continued to show declines during the test as well. During the 3-d period leading up to the test, the water-level decline was approximately 0.04 ft. This change in water level probably was caused by the startup and continuous operation of PM-4.

Water-level perturbations (diurnal in places) having a magnitude of a few hundredths of a foot appeared throughout the monitoring period. It was believed that these were Earth tide effects.

Pumping R-44 screen 1 appeared to induce slight drawdown in R-45 screen 2. The magnitude of the effect was estimated to be about 0.02 ft. Figure 13 shows an expanded-scale plot of the apparent R-45 screen 2 hydrograph along with a straight line of fit for visual reference. The effect caused by pumping R-44 screen 1 was slight, but distinct.

Pumping R-44 screen 2 caused a greater effect in R-45 screen 2 than did pumping R-44 screen 1. The drawdown in R-45 screen 2 was approximately 0.06 ft during the 24-h constant-rate pumping test conducted in R-44 screen 2.

There was a prominent water-level rise in R-45 screen 2 from late February 20 to the start of the 24-h test on R-44 screen 2 on February 21. This was similar to the water-level rise seen in R-44 screen 2 during the same period. This response was absent from the R-45 screen 1 data. Thus, the distinct water-level increase from this period was observed in both R-44 screen 2 and R-45 screen 2 but was absent from screen 1 in both wells. It was suspected that Earth tides may have caused these perturbations in the water level because no other cause could be identified. The other such rise seen in R-44 screen 2 from February 18 to 19 (Figures C-7.0-4 and C-7.0-5) was not evident in R-45 screen 2.

Figure C-7.0-14 shows a plot of the R-45 screen 2 apparent hydrograph along with operating times for production wells PM-3, PM-5, and O-4. There was no discernible correlation between production well cycling and water-level fluctuations in R-45 screen 2. It appeared that continuous operation of PM-4 beginning on February 11 caused the only identifiable water-level changes in R-45 screen 2.

Figure C-7.0-15 shows the hydrograph obtained from well R-11 located in Sandia Canyon less than half a mile north of R-44. The data were recorded using the permanently installed vented transducer, so the hydrograph fluctuated with barometric pressure rather than showing the more flat-line response typical of nonvented transducers. The times of the pumping tests on R-44 screens 1 and 2 are included on the graph for reference.

Visual examination of the hydrograph and barometric pressure curve showed that they nearly coincided. There was, however, a clear downward water-level trend from the start of monitoring on February 13 to about February 19. This was evidenced by the hydrograph lying above the barometric pressure curve initially and gradually approaching it from above. Beginning February 19, the hydrograph and barometric pressure curve pretty much coincided. It was likely that the decline in water level from February 13 to February 19 was caused by startup and operation of PM-4, which began on February 11.

Because the barometric pressure fluctuations in the hydrograph were large, it was necessary to correct the water-level data by removing the barometric effect. This was done in two ways. One procedure involved correcting the data using BETCO (barometric and Earth tide correction) software, a mathematically complex correction algorithm that uses regression deconvolution (Toll and Rasmussen 2007, 104799) to modify the data. The BETCO correction not only removes barometric pressure effects, but Earth tides as well. The BETCO corrected data are shown in Figure 15.

A visual examination of the corrected hydrograph showed minor perturbations on the order of a few hundredths of a foot, but no identifiable response to pumping either screen 1 or screen 2 in R-44.

A second correction approach was applied to the hydrograph data by correcting directly for the change in barometric pressure assuming 100% barometric efficiency and immediate response. Figure C-7.0-16 shows the hydrograph corrected in this manner. The BETCO correction was retained on the graph for comparison.

The direct correction method seemed to produce better results for R-11. The corrected hydrograph reflected the small, steady drop in level from February 13 to 19 caused by operation of PM-4. The water-level decline attributable to pumping PM-4 was about 0.06 ft over a 6-d period. Then, beginning February 19, the corrected hydrograph was nearly flat. Visual examination of the hydrograph showed no correlation between water-level fluctuations and the pumping of either screen in R-44.

Figure C-7.0-17 shows the hydrograph obtained from R-13 located roughly 980 ft east of R-44. Again the times of the R-44 pumping tests and the BETCO hydrograph correction are included on the graph.

Visual examination of the hydrograph and barometric pressure curve showed that they nearly coincided. Similar to the R-11 response, there was a clear downward water-level trend from the start of monitoring on February 13 to about February 20. This was evidenced by the hydrograph lying above the barometric pressure curve initially and gradually approaching it from above. The initial gap between the curves was wider than in R-11, indicating a more rapid water-level decline in R-13 caused by the operation of PM-4.

Beginning February 20, the hydrograph and barometric pressure curve coincided, except for a departure that occurred during the R-44 screen 2 pumping test. This indicated a possible response to pumping screen 2. Indeed, the BETCO correction showed a clear pumping response to the screen 2 test of approximately 0.06 ft. The BETCO plot suggested the lack of a response, however, to the pumping test conducted on R-44 screen 1. Finally, the BETCO plot indicated roughly 0.15 ft of water-level decline due to PM-4 pumping over roughly a 7-d period.

A second correction was performed, this time using the direct approach of correcting for barometric pressure only, assuming 100% barometric efficiency and immediate response. Figure C-7.0-18 shows the resulting corrected hydrograph. The BETCO hydrograph was retained on the figure for comparison purposes.

Similar to the BETCO plot, the corrected hydrograph in Figure C-7.0-18 showed a clear response to pumping R-44 screen 2. Unlike the BETCO plot, however, the corrected hydrograph suggested a possible subtle response to pumping screen 1. This was evidenced by the increase in the slope of the hydrograph during the pumping period followed by a flattening (cessation of the downward background trend) during recovery. To clarify this, an expanded-scale plot of the corrected hydrograph was prepared. Figure C-7.0-19 shows the expanded-scale plot of the R-13 corrected hydrograph along with a straight line of fit for visual reference. The resulting data indicated a possible pumping effect from R-44 screen 1 of about 0.03 ft. Because of the small magnitude of the effect and the fact that the BETCO correction removed it altogether, it is possible that it was an Earth tide or delayed barometric effect rather than a response to pumping R-44 screen 1.

A final observation from the corrected hydrograph in Figures C-7.0-18 and C-7.0-19 was the abrupt rise in water level from late February 20 to early February 21. This was similar to that seen in R-44 screen 2 (Figures C-7.0-4 and C-7.0-5) and R-45 screen 2 (Figures C-7.0-11 through C-7.0-14). This effect was absent, however, on the BETCO hydrograph correction (Figures C-7.0-17 and C-7.0-18). Because the BETCO algorithm removes Earth tide effects, this may be evidence that this prominent feature in all three wells was indeed caused by Earth tides.

Figure C-7.0-20 shows the hydrograph obtained from R-28 located roughly 1620 ft northwest of R-44. Again, the times of the R-44 pumping tests and the BETCO hydrograph correction are included on the graph.

Visual examination of the hydrograph and barometric pressure curve showed that they were nearly identical. Similar to the R-11 and R-13 responses, there was a clear downward water-level trend from the start of monitoring on February 13 to about February 20. This was evidenced by the hydrograph lying above the barometric pressure curve initially and gradually approaching it from above. As in the other wells, this background trend was likely caused by startup and continuous operation of PM-4.

Beginning February 20, the hydrograph and barometric pressure curve coincided, except for a departure that occurred during the R-44 screen 2 pumping test. This indicated a possible response to pumping screen 2. Indeed, the BETCO correction showed a clear pumping response to the screen 2 test of approximately 0.03 ft. The BETCO plot suggested the lack of a response, however, to the pumping test conducted on R-44 screen 1. Finally, the BETCO plot indicated roughly 0.14 ft of water-level decline due to PM-4 pumping over roughly a 7-d period.

A second correction was performed, this time using the direct approach of correcting for barometric pressure only, assuming 100% barometric efficiency and immediate response. Figure C-7.0-21 shows the resulting corrected hydrograph. The BETCO hydrograph was retained in the figure for comparison purposes.

Similar to the BETCO plot, the corrected hydrograph in Figure C-7.0-21 showed a clear response to pumping R-44 screen 2. Unlike the BETCO plot, however, there was a hint of a possible response to pumping R-44 screen 1. This was evidenced by a slight increase in the slope of the hydrograph during the pumping period followed by a reduction in slope during recovery. To clarify these subtle effects, an expanded-scale plot of the corrected hydrograph was prepared. Figure C-7.0-22 shows the expanded-scale plot of the R-28 hydrograph along with a straight line of fit for visual reference. The data indicated a possible tiny response to pumping R-44 screen 1 of no more than about 0.01 ft. (Perhaps a logarithmic-shaped reference curve of some sort would have been more appropriate than the straight line shown on Figure C-7.0-22, but the conclusion of a deflection in the hydrograph would have been the same.) Because the effect was so small and the BETCO correction removed it altogether, it is possible that it was caused by something other than the R-44 screen 1 pumping test such as Earth tides or delayed barometric response. However, the striking coincidence of occurring during the screen 1 pumping test coupled with similar, though larger, responses observed in the other monitored wells, made it possible that the screen 1 pumping effects reached R-28.

A final observation from the corrected hydrograph in Figure C-7.0-22 was the data segment from late February 20 to early February 21. Contrary to the antecedent decline in water levels, there was a small rise in level during this period—a subdued version of the pronounced water-level rise seen in R-13, R-44 screen 2, and R-45 screen 2 probably caused by Earth tides.

C-8.0 R-44 SCREEN 1 DATA ANALYSIS

This section presents the data obtained from the R-44 screen 1 pumping tests and the results of the analytical interpretations. Data are presented for drawdown and recovery for trials 1 and 2 and the 24-h constant-rate pumping test.

R-44 Screen 1 Trial 1

Figure C-8.0-1 shows a semilog plot of the trial 1 drawdown data. The early data showed exaggerated drawdown because the pumping rate was elevated as the drop pipe was being filled for the first time and the pump operated against reduced head.

The middle data on the plot showed drawdown changes associated with erratic pump operation. The generator used for the trials tests on R-44 was defective, showing below normal alternating current output frequency as well as variable current frequency and voltage.

The late data showed drawdown changes associated with discharge rate adjustments made using the flow control valve at the surface. The average discharge rate during trial 1 was 19.2 gpm, while the rate over the last half of the test was 19.5 gpm. The many discharge rate changes during trial 1 precluded analysis of the drawdown data.

Figure C-8.0-2 shows a semilog plot of the trial 1 recovery data. The transmissivity value computed from the early data was 4210 gpd/ft. Based on the screen length of 10 ft, the computed hydraulic conductivity was 421 gpd/ft², or 56.3 ft/d.

The slope of the data trace began declining at just a few seconds, and within minutes the slope became essentially flat. Figure C-8.0-3 shows an expanded-scale plot of the late-recovery data.

The flattening of the recovery curve showed the effects of a complex combination of vertical growth of the cone of impression (partial penetration), leakage from below, and delayed drainage of the unconfined aquifer. For illustration purposes, a transmissivity was computed for the late data yielding a value of 270,000 gpd/ft. This was likely not an actual transmissivity but rather an artifact of the delayed yield and leakage. It does suggest, however, the possibility of a large aquifer transmissivity at the R-44 location.

R-44 Screen 1 Trial 2

Figure C-8.0-4 shows a semilog plot of the trial 2 drawdown data. The discharge rate for trial 2 was 20.0 gpm. The transmissivity value computed from the early data was 3550 gpd/ft, making the computed hydraulic conductivity 355 gpd/ft², or 47.5 ft/d. Note that the early data showed the effects of a minimal amount of antecedent drainage of the drop pipe through leaky coupling joints.

Later data in Figure C-8.0-4 showed erratic pumping water levels associated with discharge rate fluctuations induced by the inconsistent operation of the electric generator.

Figure C-8.0-5 shows an expanded-scale plot of the trial 2 drawdown data. At this scale, the plot showed more clearly the erratic drawdown induced by the variable generator output. The discharge rate variations precluded analysis of the late data. Nevertheless, the late data showed the effects of the combination of delayed yield and vertical expansion of the cone of depression.

Figure C-8.0-6 shows a semilog plot of the trial 2 recovery data. The transmissivity value computed from the early data was 3350 gpd/ft, making the computed hydraulic conductivity 355 gpd/ft², or 47.5 ft/d. The late-recovery data showed flattening associated with a combination of delayed yield, partial penetration and leakage.

Figure C-8.0-7 shows an expanded-scale plot of the late trial 2 recovery data.

The curve appeared to flatten completely (delayed yield) and did not support calculation of a representative transmissivity value. The severe data scatter coupled with the tiny changes in head precluded a rigorous analysis of the late data. The 44-h duration of the recovery period should have been

long enough to exhaust the delayed drainage effect, implying the possibility of a high transmissivity for the regional aquifer at this location.

C-8.1 R-44 Screen 1 24-H Constant-Rate Pumping Test

Figure C-8.1-1 shows a semilog plot of the drawdown data recorded during the 24-h constant-rate pumping test conducted at a discharge rate of 24.2 gpm. The early data showed that some antecedent drainage of the drop pipe had occurred during the background monitoring period. The magnitude of the drawdown spike during the first minute of pumping allowed estimating the early pumping rate before refilling the void in the drop pipe, roughly 28 gpm. Using this discharge rate estimate, the transmissivity computed from the initial drawdown data was 3860 gpd/ft, making the computed hydraulic conductivity 3817 gpd/², or 42.4 ft/d.

The late data showed a reduction in drawdown over time. There was no obvious explanation for the odd occurrence because the measured discharge rates were constant, especially over the last half day of pumping. It is possible that minor sediment removal with the pumped water could have increased the well efficiency somewhat during the test.

Figure C-8.1-2 shows the recovery data measured following the 24-h constant-rate pumping test. The transmissivity calculated from the early data was 3510 gpd/ft, making the hydraulic conductivity 351 gpd/ft², or 46.9 ft/d.

As with all of the data plots, the late-recovery data showed severe flattening associated with delayed yield and vertical expansion of the cone of impression. Figure C-8.1-3 shows an expanded-scale plot of the late-recovery data.

Again, the latest slope in Figure C-8.1-3 did not support calculation of a meaningful transmissivity value because of possible lingering delayed yield effects as well as the broad data scatter. Nevertheless, the data suggested the possibility of a transmissive regional aquifer at the R-44 location.

Packer Deflation

Following the 24-h recovery period, the packer was deflated in preparation for pulling the pump. When this was done, water above the packer that had leaked through coupling joints in the drop pipe bypassed the packer and was delivered to the pressure transducer while the water drained back into the well and formation. This caused a pressure increase that was recorded by the transducer.

Figure C-8.1-4 shows the resulting head buildup and decay that occurred when the packer was deflated. Data were recorded at 1-min intervals so the maximum head buildup was not revealed in the data set. The high specific capacity of R-44 meant that a substantial volume of water could have flowed into the screen zones before the first head measurement in Figure C-8.1-4. The head data confirmed that pipe joints had leaked throughout the R-44 screen 1 pumping tests.

R-44 Screen 1 Specific Capacity Data

Specific capacity data were used along with well geometry to estimate a lower-bound conductivity value for the R-44 screen 1 zone for comparison to the pumping test values. In addition to specific capacity, other input values used in the calculations included the assumed aquifer thickness of 67 ft (from the static water level to the midpoint of the blank pipe section between screens 1 and 2), a storage coefficient of 0.1 and a borehole radius of 0.51 ft. The calculations are somewhat insensitive to the assigned aquifer thickness, as long as the selected value is substantially greater than the screen length.

R-44 screen 1 produced 24.2 gpm with a drawdown of 4.27 ft after 24 h of pumping for a specific capacity of 5.67 gpm/ft. Applying the Brons and Marting method to these inputs yielded a lower-bound hydraulic conductivity value for the screened interval of 432 gpd/ft², or 57.7 ft/d. Because the calculation method did not factor in the effects of leakage, it was possible that the computed value could be overestimated. Indeed, the value was somewhat greater than the values obtained from the early pumping test data but similar enough that it was not considered an unreasonable result. Overall, it provided corroboration of the pumping test values and suggested a hydraulically efficient completion.

R-44 Screen 1 Summary

Table 8.1-1 summarizes the hydraulic conductivity values obtained from the R-44 screen 1 pumping test analyses. The average hydraulic conductivity computed from the recovery data was 50.2 ft/d. The recovery average was used because of the effects of antecedent drop pipe drainage on the drawdown data.

The specific capacity obtained from screen 1 suggested a lower-bound hydraulic conductivity of 57.7 ft/d. However, that value did not factor in the effects of leakage that occurred in screen 1 and, thus, was considered consistent with the pumping test values. The results suggested a highly efficient screened interval.

Within seconds of startup or shutdown, vertical expansion of the cone of depression (leakage and partial penetration) and delayed yield affected the pumping and recovery data. The late data suggested an enormous transmissivity for the regional aquifer at the R-44 location, perhaps in excess of 100,000 gpd/ft.

C-9.0 R-44 SCREEN 2 DATA ANALYSIS

This section presents the data obtained from the R-44 screen 2 pumping tests and the results of the analytical interpretations. Data are presented for drawdown and recovery for trials 1 and 2 and the 24-h constant-rate pumping test.

Analysis of the screen 2 data was challenging because of the lack of early data from most of the tests. A programming oversight made in setting up the transducer data collection scheme, coupled with slightly varying clock speeds of the wristwatch used during the tests and the transducers, led to losing the very early data in all but one of the tests.

R-44 Screen 2 Trial 1

Figure C-9.0-1 shows a semilog plot of the drawdown data collected from trial 1 conducted at a discharge rate of 23.9 gpm. The early data showed exaggerated drawdown because the pumping rate was elevated as the drop pipe was being filled for the first time and the pump operated against reduced head.

The varying pumping rate associated with filling the drop pipe precluded analysis of the early drawdown data.

The data following filling of the drop pipe were plotted on an expanded scale as shown in Figure C-9.0-2. The slope of the graph became continuously flatter throughout the trial test. This was caused by a combination of vertical expansion of the cone of depression and leakage from the screen 1 zone. It is also possible that the data included indirect effects of delayed yield of the overlying unconfined screen 1 interval

The latest data on the graph supported a transmissivity calculation of 48,500 gpd/ft. This likely represented the thickness of sediment corresponding to the depth of the cone of depression at that particular time. The total aquifer transmissivity is likely greater than indicated by this calculation because

the cone of depression was probably still expanding vertically and the slope of the graph likely would have continued flattening at later time. Note also that the substantial data scatter added uncertainty to the calculation. In fact, the amount of data scatter exceeded the drawdown change on which the analysis was based.

Figure C-9.0-3 shows the recovery data collected following shutdown of the trail 1 pumping test. The transmissivity computed from the early data on the graph was 3430 gpd/ft. Dividing this value by the screen length of 9.9 ft yielded a hydraulic conductivity estimate of 346 gpd/ft², or 46.3 ft/d. However, the earliest recovery data were not collected. Note that the first data point corresponded to a residual drawdown of less than a foot out of more than 18 ft of drawdown at the end of the pumping period. It was suspected that the recovery cone of impression had expanded vertically well beyond the length of the screened interval to some greater effective thickness and that the hydraulic conductivity was likely substantially less than computed from Figure C-9.0-3. Subsequent data, presented below, corroborated this idea.

Figure C-9.0-4 shows an expanded-scale plot of the trial 1 recovery data. The slope of the recovery curve continued to flatten throughout the monitored period. The latest data supported a transmissivity calculation of 124,000 gpd/ft.

There was uncertainty in this transmissivity calculation because of the tiny water-level changes and the substantial data scatter. Furthermore, there was no way to know the height of the cone of impression corresponding to the analysis or whether delayed yield from the upper aquifer zone was affecting the data. The analysis did suggest, however, a large aquifer transmissivity.

R-44 Screen 2 Trial 2

Figure C-9.0-5 shows a semilog plot of the drawdown data collected from trial 2 conducted at a discharge rate of 24.0 gpm. The data from the first few seconds of pumping showed exaggerated drawdown associated with minor antecedent drainage of a portion of the drop pipe through a leaky coupling joint. This precluded capturing the very early data for analysis.

The early data following refilling of the void in the drop pipe were plotted on an expanded scale as shown in Figure C-9.0-6. The transmissivity computed from the graph was 2900 gpd/ft. Diving this value by the screen length of 9.9 ft yielded a hydraulic conductivity estimate of 293 gpd/ft², or 39.2 ft/d.

It was likely that the cone of depression had already expanded well beyond the thickness of screened sediment so the hydraulic conductivity values computed based on the screen length of 9.9 ft were considered overestimates of the actual value. Note that the earliest data used in the analysis already showed nearly 16 ft of drawdown. In other words, the snapshot of the early data associated with initial lateral expansion of the cone of depression was masked by the discharge rate fluctuations caused by changing head conditions as the void in the drop pipe refilled. By the time postrefill data were collected, the cone of depression had expanded vertically.

Figure C-9.0-7 shows an expanded-scale plot of the late drawdown data from trial 2. The transmissivity value obtained from the latest slope on the graph was 146,000 gpd/ft. There was uncertainty in the calculated value because of wide data scatter and ongoing vertical expansion of the cone of depression as well as leakage and possible delayed yield effects from the overlying zone.

Figure C-9.0-8 shows the recovery data recorded following the trial 2 test on R-44 screen 2. The data set included earlier data than any other data set obtained in the testing effort, as evidenced by the fact that the residual drawdown was still at about 7 ft when data collection began. This data set allowed obtaining a reasonably representative snapshot of the early-recovery response.

The transmissivity computed from the early-recovery data was 820 gpd/ft, much smaller than previous values. Based on the screen length of 9.9 ft, the computed hydraulic conductivity was 82.8 gpd/², or 11.1 ft/d. This is likely a good representation of the hydraulic conductivity of the screened sediments. It implied that the larger values obtained previously were biased by vertical expansion of the drawdown or recovery cone.

Figure C-9.0-9 shows an expanded-scale plot of the late trial 2 recovery data. As with previous graphs of late drawdown or recovery data, the slope of the data trace flattened continuously throughout the recovery period, becoming essentially horizontal at the end of the monitoring period.

The continuous flattening resulted from partial penetration effects including both vertical expansion of the cone of impression and leakage from the upper zone, including possible delayed yield effects. No transmissivity value was calculated from the graph in Figure C-9.0-9, as it would have supported computation of any arbitrarily large value, depending on which portion of the curve was used for constructing the line of fit. As stated earlier, the late-time response was consistent with a large transmissivity for the regional aquifer at the R-44 location.

C-9.1 R-44 Screen 2 24-H Constant-Rate Pumping Test

Figure C-9.1-1 shows a semilog plot of the drawdown data recorded during the 24-h constant-rate pumping test conducted at a discharge rate of 23.9 gpm. The early data showed that antecedent drainage of the drop pipe had occurred during the background monitoring period.

The effect of variable discharge rates while filling the void in the drop pipe precluded analysis of the early data from the drawdown curve.

Figure C-9.1-2 shows an expanded-scale plot of the drawdown data following refilling of the drop pipe. The data set was quite noisy and did not support a useful analysis.

The drawdown spike at a time of 3 min was accompanied by a noise at the well head that sounded like air moving through the discharge piping. It was not known if these two events were cause-and-effect or merely coincidental. Generally, air is not entrained in the drop pipe when it drains, because the drained void is typically under vacuum conditions. However, if there were two leaky coupling joints between adjacent check valves, it is possible that water could have drained from the lower one while air was pulled into the upper one.

The abrupt drop in pumping water level that occurred between 600 and 1000 min was caused by temporarily pumping the discharge water to a lower elevation. Against the reduced head, the pumping rate increased by about 1%.

All other perturbations in the drawdown data were caused by uncontrollable discharge rate fluctuations associated with submersible pump and electric generator operation. Such variations in flow rate are not unusual.

Figure C-9.1-3 shows the recovery data measured following the 24-h constant-rate pumping test. The transmissivity calculated from the early data was 1760 gpd/ft, making the computed hydraulic conductivity 178 gpd/ft², or 23.8 ft/d.

Because the residual drawdown corresponding to the first data point on Figure C-9.1-3 was only 1.4 ft, most of the recovery had already occurred and it was likely that the earliest data, required to identify the properties of the screened sediments, were not collected. Indeed, the computed hydraulic conductivity was

substantially greater than the value obtained from the trial 2 recovery data. It was likely that the cone of impression had expanded vertically beyond the screened interval before the first data point was recorded.

As with all of the data plots, the late-recovery data showed severe flattening associated with vertical expansion of the cone of impression, leakage and perhaps delayed yield. Figure C-9.1-4 shows an expanded-scale plot of the late-recovery data.

The slope of the data plot decreased continuously, eventually becoming and remaining essentially flat. It was hypothesized that at late time, the water level would eventually reach the initial static level. A line of fit was constructed to pass through the late data and the upper le corner of the graph (corresponding to zero residual drawdown at infinite time). The transmissivity computed from the artificially constructed line of fit was 39,100 gpd/ft.

There was uncertainty in the computed transmissivity value. The scatter in the data set exceeded the magnitude of water-level change on which the analysis was based. Further, any change in the background water level would have meant that targeting zero residual drawdown in the analysis was incorrect.

Regardless of the approach used to analyze the late-recovery data, the flat slope of the late data implied a high transmissivity for the regional aquifer at the R-44 location.

R-44 Screen 2 Specific Capacity Data

Specific capacity data were used along with well geometry to estimate a lower-bound conductivity value for the R-44 screen 2 zone for comparison to the pumping test values. In addition to specific capacity, other input values used in the calculations included the assumed an arbitrarily assigned aquifer thickness of 200 ft, storage coefficient values of 0.01 and 0.001 (for leaky-confined and confined conditions, respectively), and a borehole radius of 0.51 ft.

R-44 screen 2 produced 23.9 gpm with a drawdown of 17.72 ft after 24 h of pumping for a specific capacity of 1.35 gpm/. Applying the Brons and Marting method to these inputs yielded lower-bound hydraulic conductivity values for the screened interval of 111 gpd/², or 14.9 ft/d, for leaky-confined conditions and 113 ft², or 15.1 ft/d, for confined conditions.

These values were greater than the hydraulic conductivity estimate of 11.1 ft/d obtained from the trial 2 recovery data. However, the calculations were based on the assumption of homogeneous conductivity. It was known that the overlying sediments (screen 1) have a hydraulic conductivity around 50 ft/d. In addition, the rapid and severe flattening of all of the screen 2 drawdown and recovery curves was consistent with substantial transmissivity adjacent to the screened horizon. A reasonable explanation of the calculation results was that the screen 2 interval has a lower permeability than the adjacent overlying and/or underlying sediments. The greater permeability of the surrounding sediments (above and/or below the screen 2 interval) enhanced the specific capacity of screen 2. In this light, the computed lower-bound hydraulic conductivity based on the specific capacity performance of screen 2 appeared reasonable and consistent with the trial 2 recovery analysis.

R-44 Screen 2 Summary

Failure to collect very early data from most of the tests on screen 2 made determining formation properties a challenge. The best estimate of the hydraulic conductivity of the screened sediments was 11.1 ft/d from the trial 2 recovery data. The surrounding sediments (above and/or below the screened interval), however, appeared to have a substantially greater conductivity.

The specific capacity obtained from screen 2 suggested a lower-bound hydraulic conductivity of about 15 ft/d. However, that value did not factor in the effects of higher permeability adjacent sediments. In that light, the results were consistent with the estimated hydraulic conductivity of the screened interval. The results also implied an efficient completion.

Within seconds of startup or shutdown, vertical expansion of the cone of depression (leakage and partial penetration) and possibly delayed yield from the upper zone sediments affected the pumping and recovery data. The late data suggested an enormous transmissivity for the regional aquifer at the R-44 location, perhaps in excess of 100,000 gpd/ft.

C-10.0 LEAKANCE/RESISTANCE OF SEDIMENTS BETWEEN SCREENS 1 AND 2

Data from the pumping tests were used to estimate the leakance of the sediments separating R-44 screen 1 from screen 2. Each of the 24-h tests supported estimation of this parameter.

Pumping R-44 screen 1 at 24.2 gpm produced approximately 0.05 ft of drawdown in screen 2, while pumping screen 2 at 23.9 gpm resulted in about the same 0.05 ft of drawdown in screen 1. These responses to pumping were simulated analytically using Equations C-10 and C-11, assuming a uniform vertical anisotropy ratio. For each pumping test, the vertical anisotropy was adjusted until the observed drawdown in the nonpumped zone matched the field observation. The actual sediments are layered, not homogeneous, so the calculations just supported determination of an overall effective vertical resistance to flow.

The following assumptions were used in the calculations:

- aquifer thickness = 300 ft
- hydraulic conductivity = 50 ft/d
- storage coefficient ranged from 0.002 to 0.05.
- pumping rate = 24.2 gpm/23.9 gpm.
- static water level = 879 ft
- screen 1: 895 to 905 ft
- screen 2: 985.3 to 995.2 ft
- pumping time = 1440 min

The assumed hydraulic conductivity value of 50 ft/d matched the value obtained from the screen 1 pumping test. The screen 2 test produced a lower value, but the overall response of screen 2 was consistent with a highly transmissive aquifer. It was possible that screen 2 was set in lower permeability sediments than the aquifer average. Although the accuracy of the estimate was uncertain and the actual aquifer thickness assignment was arbitrary, the calculations were useful to provide a sense of the vertical permeability of the aquifer.

Using the above inputs, Equation C-10 was solved for anisotropy ratio by adjusting the ratio until the drawdown at 1440 min was equal to 0.05 ft. The computations were repeated for a few values of storage coefficient ranging from 0.002 to 0.05. Figure C-10.0-1 shows the computed relationship between storage coefficient and vertical anisotropy ratio. Virtually identical results were obtained for the two pumping tests.

The figure showed that there was insufficient data to determine the vertical anisotropy ratio accurately. Its value varied substantially as a function of storage coefficient and therefore its estimate was only as accurate as the estimate of storage coefficient.

For example, according to Figure C-10.0-1 for an assumed storage coefficient value of 0.01, the computed vertical anisotropy ratio was 0.015. Based on the assumed hydraulic conductivity of 50 ft/d, this made the estimated vertical permeability $0.015 \times 50 = 0.75$ ft/d. The corresponding leakance of the 80 ft of sediments separating the two screens was 0.75/80 = 0.00938 inverse days and the computed resistance was 1/0.00938 = 107 d. These calculations showed moderate vertical permeability, indicating the absence of a real aquitard. The results suggested moderate vertical movement of groundwater in the vicinity of R-44 screens 1 and 2.

These results implied a fairly conductive separating layer between screen 1 and screen 2, similar to formation characteristics at R-43, but different than what has been observed at other locations on the Plateau where the head separation between the uppermost screens in multi-screened wells is greater than observed here. As a comparison, similar analysis at R-35a and R-35b yielded hydraulic resistance on an order of magnitude greater than computed for R-44, while analysis of R-10 screens 1 and 2 data showed resistance more than two orders of magnitude greater. Note that part of the greater resistance at the other locations is attributable to the greater distance between the well screens. R-44 screens 1 and 2 are 80 ft apart, whereas the separation distance at R-35a/b is about 167 ft (accounting for elevation difference between the two wells) and that at R-10 is about 144 ft. From screen center to screen center, the downward gradients in R-35 a/b and R-10 are 0.031 ft and 0.083 /, respectively, compared with 0.0022 in R-44. Although computations like this have not been made for R-33, it is likely that the hydraulic resistance between screens 1 and 2 at that location is similar to what was determined for R-10 based on the large head difference between the screens in R-33. Thus, compared with other locations on the Plateau, the potential for vertical groundwater movement at R-44 (as well as R-43) is relatively favorable.

C-11.0 SUMMARY

Constant-rate pumping tests were conducted on R-44 screens 1 and 2 in Mortandad Canyon. The tests were conducted to gain an understanding of the hydraulic characteristics of the aquifers in which the screens were installed as well as the intervening sediments between the screens. Additionally, several surrounding wells were monitored to check for hydraulic cross connection to R-44.

Numerous observations and conclusions were drawn for the tests as summarized below.

The static water level in R-44 screen 1 was only 0.2 ft higher than in screen 2, suggesting minimal vertical hydraulic resistance of the intervening sediments. Consistent with this idea, analysis of interference effects between screen 1 and screen 2 (about 0.05 ft after 24 h of pumping 24 gpm) suggested moderate leakance.

All monitored wells and screen zones (R-44 screens 1 and 2, R-45 screens 1 and 2, R-11, R-13, and R-28) showed immediate water-level response to barometric pressure with a barometric efficiency of essentially 100%.

There was no correlation between water levels in any of the monitored wells and cycling of production wells PM-3, PM-5, and O-4. PM-4, on the other hand, which was started up a few days before the test program and ran continuously throughout, induced a small but steady drawdown trend in each of the monitored wells.

In addition to screens 1 and 2 affecting one another when pumping was performed, most (but not all) of the monitored screen zones showed slight pumping response. Table C-11.0-1 summarizes the pumping effects induced by testing R-44 screens 1 and 2, as well as that caused by continuous operation of PM-4.

Leaky threaded joints in the drop pipe used to hang the submersible test pump allowed drainage of a portion of the pipe between pumping events. Pumping against reduced head briefly until the void in the drop pipe was refilled resulted in chaotic discharge rate changes at the onset of pumping, corrupting much of the early drawdown data and rendering it unusable for determining aquifer properties. The early-recovery data, however, were usable. The leaky joints were likely attributable to a combination of worn threads, improperly manufactured threads, and the need to avoid over-tightening the threads to avoid galling.

The pumping test data indicated a hydraulic conductivity for the screen 1 sediments of about 50 ft/d.

Specific capacity analysis showed that screen 1 produced 24.2 gpm with 4.27 ft of drawdown, for a specific capacity of 5.67 gpm/ft. The lower-bound hydraulic conductivity computed from this information was 57.7 ft/d. Considering that this calculation did not consider the effects of leakage and therefore was probably overestimated, it provided reasonable corroboration of the pumping test hydraulic conductivity value.

The pumping test data indicated a hydraulic conductivity for the screen 2 sediments of about 11 ft/d.

Specific capacity analysis showed that screen 2 produced 23.9 gpm with 17.72 ft of drawdown, for a specific capacity of 1.35 gpm/ft. The lower-bound hydraulic conductivity computed from this information was about 15 ft/d. Considering that this calculation was based on homogeneous conditions and did not consider the effects of the greater permeability of adjacent sediments and therefore was probably overestimated, it provided reasonable corroboration of the pumping test hydraulic conductivity value.

All of the pumping tests showed immediate flattening of the drawdown and/or recovery curves. This reflected the effects of a combination of delayed yield and partial penetration (vertical expansion of the cone of depression). The fact that the drawdown and recovery curves remained flat at late time suggested a very large aquifer transmissivity, perhaps as great as 100,000 gpd/ft. At late time, the change in water level was within the "noise" level and accurate quantification of aquifer transmissivity was not possible.

C-12.0 REFERENCES

The following list includes all documents cited in this appendix. Parenthetical information following each reference provides the author(s), publication date, and ER ID. This information is also included in text citations. ER IDs are assigned by the Environmental Programs Directorate's Records Processing Facility (RPF) and are used to locate the document at the RPF and, where applicable, in the master reference set.

Copies of the master reference set are maintained at the NMED Hazardous Waste Bureau and the Directorate. The set was developed to ensure that the administrative authority has all material needed to review this document, and it is updated with every document submitted to the administrative authority. Documents previously submitted to the administrative authority are not included.

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Figure C-7.0-1 R-44 screen 1 apparent hydrograph during R-44 screen 1 and 2 tests



Figure C-7.0-2 R-44 screen 1 rolling apparent hydrograph during R-44 screen 1 and 2 tests



Figure C-7.0-3 R-44 screen 1 apparent hydrograph during R-45 screen 1 and 2 tests



Figure C-7.0-4 R-44 screen 2 apparent hydrograph during R-44 screen 1 and 2 tests



Figure C-7.0-5 R-44 screen 2 rolling average apparent hydrograph during R-44 screen 1 and 2 tests



Figure C-7.0-6 R-44 screen 2 apparent hydrograph during R-44 screen 1 and 2 tests with County well operation



Figure C-7.0-7 R-44 screen 2 apparent hydrograph during R-45 screen 1 and 2 tests



Figure C-7.0-8 R-44 screen 2 rolling average apparent hydrograph during R-45 screen 1 and 2 tests



Figure C-7.0-9 R-45 screen 1 apparent hydrograph during R -44 screen 1 and 2 tests



Figure C-7.0-10 R-45 screen 1 apparent hydrograph during R-44 screen 1 and 2 tests with County well operation



Figure C-7.0-11 R-45 screen 2 apparent hydrograph during R-44 screen 1 and 2 tests



Figure C-7.0-12 R-45 screen 2 rolling average apparent hydrograph during R-44 screen 1 and 2 tests



Figure C-7.0-13 R-45 screen 2 apparent hydrograph during R-44 screen 1 and 2 tests—expanded scale



Figure C-7.0-14 R-45 screen 2 apparent hydrograph during R-44 screen 1 and 2 tests with County well operation



Figure C-7.0-15 R-11 hydrograph during R-44 screen 1 and 2 tests



Figure C-7.0-16 R-11 corrected hydrograph during R-44 screen 1 and 2 tests


Figure C-7.0-17 R-13 hydrograph during R-44 screen 1 and 2 tests



Figure C-7.0-18 R-13 corrected hydrograph during R-44 screen 1 and 2 tests



Figure C-7.0-19 R-13 corrected hydrograph during R-44 screen 1 and 2 tests—expanded scale



Figure C-7.0-20 R-28 hydrograph during R-44 screen 1 and 2 tests



Figure C-7.0-21 R-28 corrected hydrograph during R-44 screen 1 and 2 tests



Figure C-7.0-22 R-28 corrected hydrograph during R-44 screen 1 and 2 tests—expanded scale



Figure C-8.0-1 Well R-44 screen 1 trail 1 drawdown



Figure C-8.0-2 Well R-44 screen 1 trail 1 recovery



Figure C-8.0-3 Well R-44 screen 1 trail 1 recovery—expanded scale



Figure C-8.0-4 Well R-44 screen 1 trail 1 drawdown



Figure C-8.0-5 Well R-44 screen 1 trail 2 drawdown—expanded scale



Figure C-8.0-6 Well R-44 screen 1 trail 2 recovery



Figure C-8.0-7 Well R-44 screen 1 trail 2 recovery—expanded scale



Figure C-8.1-1 Well R-44 screen 1 drawdown



Figure C-8.1-2 Well R-44 screen 1 recovery



Figure C-8.1-3 Well R-44 screen 1 recovery—expanded scale



Figure C-8.1-4 Head buildup following packer deflation



Figure C-9.0-1 Well R-44 screen 2 trail 1 drawdown



Figure C-9.0-2 Well R-44 screen 2 trial 1 drawdown—expanded scale



Figure C-9.0-3 Well R-44 screen 2 trial 1 recovery



Figure C-9.0-4 Well R-44 screen 2 trial 1 recovery—expanded scale



Figure C-9.0-5 Well R-44 screen 2 trial 2 drawdown



Figure C-9.0-6 Well R-44 screen 2 trial 2 drawdown—early data



Figure C-9.0-7 Well R-44 screen 2 trial 2 drawdown—late data



Figure C-9.0-8 Well R-44 screen 2 trial 2 recovery



Figure C-9.0-9 Well R-44 screen 2 trial 2 recovery—expanded scale



Figure C-9.1-1 Well R-44 screen 2 drawdown



Figure C-9.1-2 Well R-44 screen 2 drawdown—expanded scale



Figure C-9.1-3 Well R-44 screen 2 recovery



Figure C-9.1-4 Well R-44 screen 2 recovery—expanded scale



Figure C-10.0-1 Correlation of storage coefficient and anisotropy

Analysis	Hydraulic Conductivity (ft/d)	
Trial 1 Recovery	56.3	
Trial 2 Drawdown	47.5	
Trial 2 Recovery	47.5	
24-H Drawdown	42.5	
24-H Recovery 46.9		
Recovery Average 50.2		

Table C-8.1-1 R-44 Screen 1 Pumping Test Results

Table C-11.0-1 R-44 Interference Effects

Drawdown (ft)			
Well Name (Screen ID)	Pump PM-4	Pump R-44 Screen 1	Pump R-44 Screen 2
R-44 Screen 1	0.03 (2.5 d)	n/a ^a	0.05
R-44 Screen 2	0.06 (2.5 d)	0.05	n/a
R-45 Screen 1	0.11 (6 d)	0.00	0.02
R-45 Screen 2	0.04 (3 d)	0.02	0.06
R-11	0.06 (6 d)	0.00	0.00
R-13	0.15 (7 d)	0.03 ^b	0.06
R-28	0.14 (7 d)	0.01 ^b	0.03

^a n/a = Not applicable.

^b Subtle effect.

Appendix D

Borehole Video Logging (on DVD included with this document)

Appendix E

Schlumberger Geophysical Logging Report (on CD included with this document)